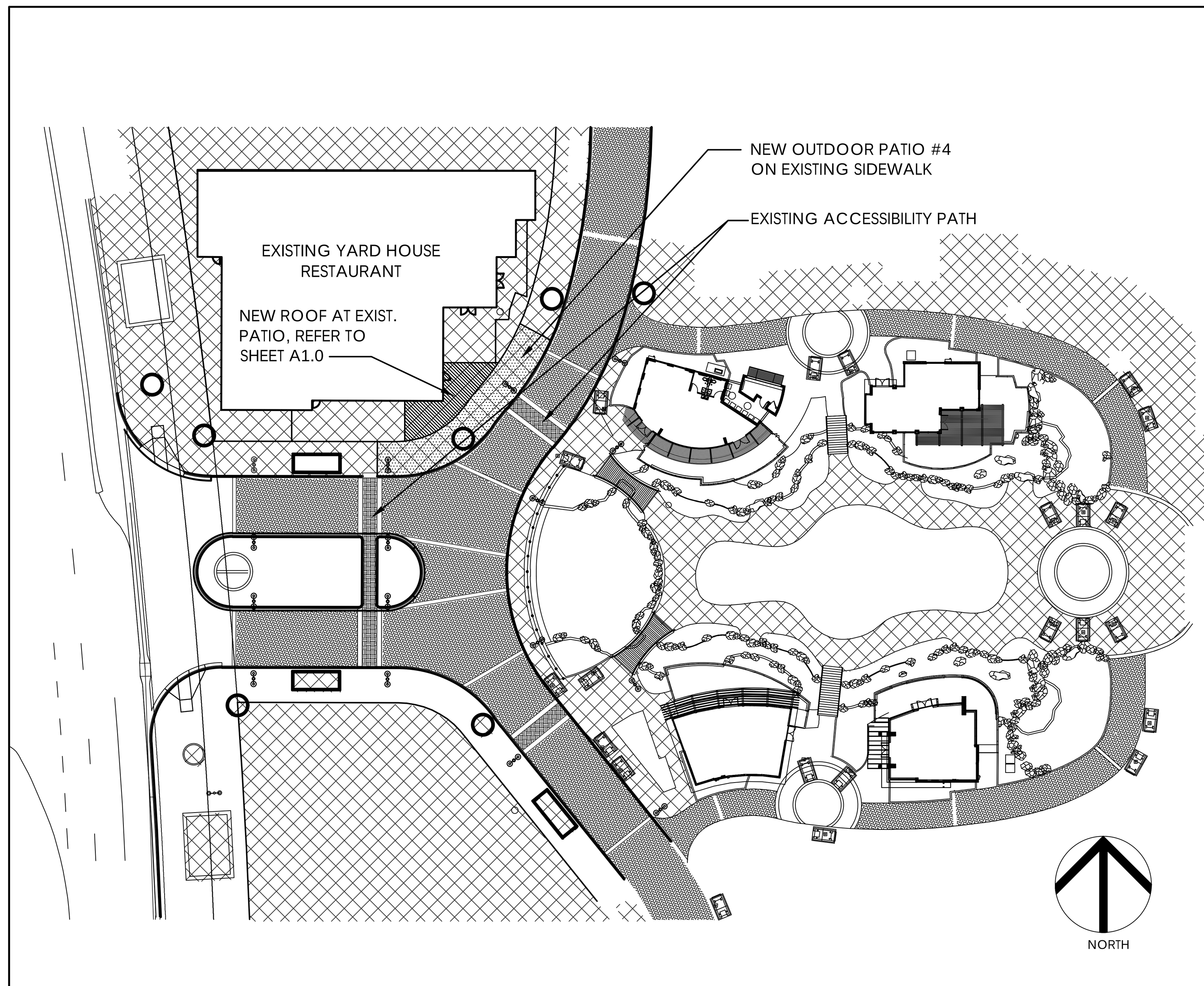
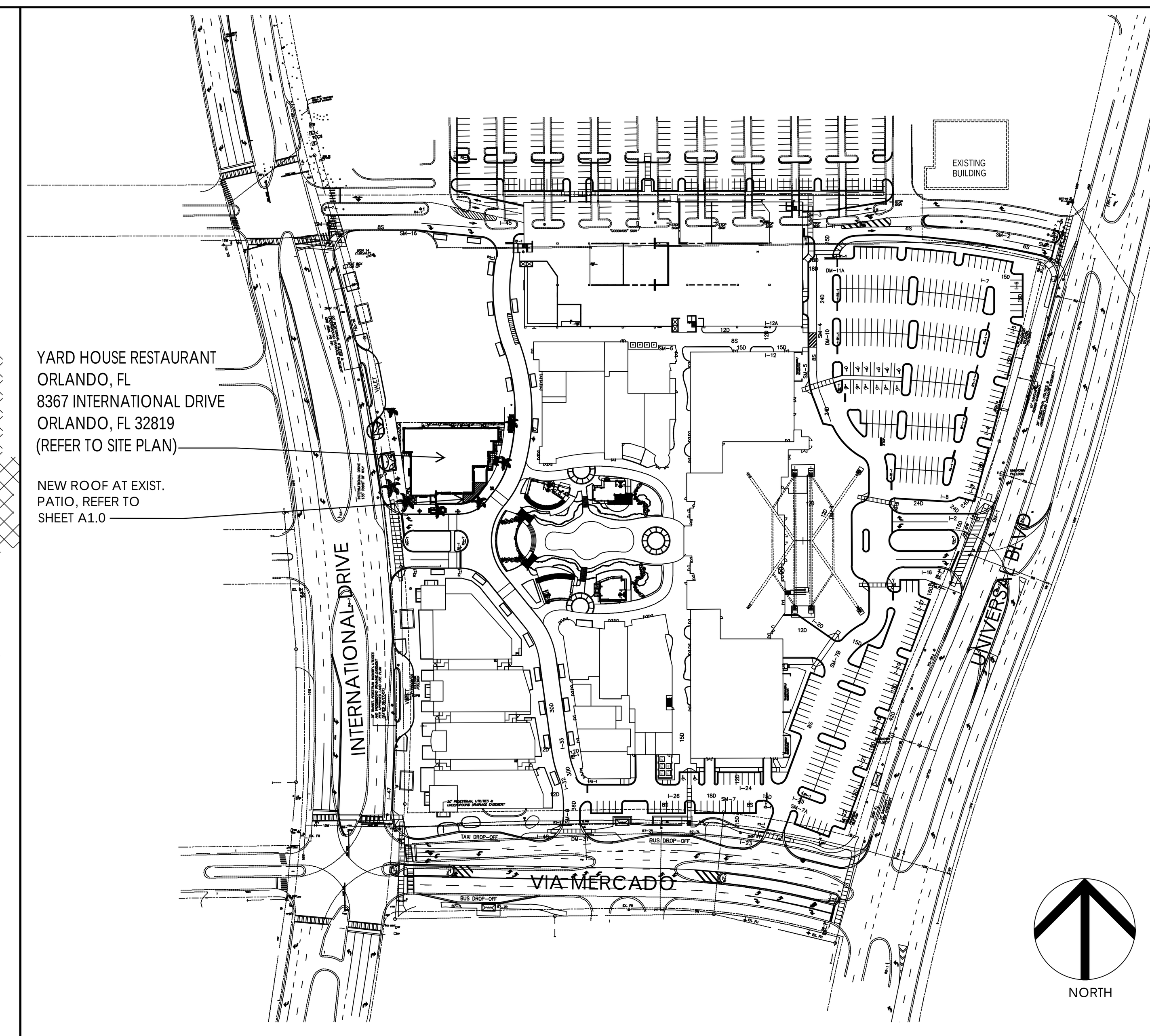
YARD HOUSE RESTAURANT LLC

NEW ROOF AT EXISTING PATIO/ PATIO ADDITION

8367 INTERNATIONAL DRIVE
ORLANDO, FL 32819



SITE PLAN 1"=40'



LOCATION PLAN

GENERAL NOTES

1 THE CONTRACTOR SHALL VERIFY EXISTING CONDITIONS WITH DOCUMENTS GIVEN AT PRE - BID MEETING AND ACCEPT PREMISES AS FOUND. OWNER ASSUMES NO RESPONSIBILITY FOR THE CONDITION OF THE EXISTING SITE OR EXISTING STRUCTURES AT THE TIME OF BIDDING OR THEREAFTER.

2 DIMENSIONS TAKE PRECEDENCE OVER DRAWINGS; DO NOT SCALE DRAWINGS TO DETERMINE ANY LOCATION. THE OWNER SHALL BE NOTIFIED IF ANY DISCREPANCY OCCURS PRIOR TO CONTINUINM WITH WORK.

3 DISCREPANCIES BETWEEN PORTIONS OF THE CONTRACT DOCUMENTS ARE NOT INTENDED. THE CONTRACTOR IS TO CLARIFY WITH THE ARCHITECT AND OWNER ANY SUCH DISCREPANCIES PRIOR TO COMMENCING WORK.

4 THE CONTRACTOR SHALL REPORT TO THE DARDEN PROJECT MANAGER ANY ERRORS, INCONSISTENCIES, OR OMISSIONS HE MAY DISCOVER. THE CONTRACTOR IS RESPONSIBLE FOR CORRECTING ANY ERRORS AFTER THE START OF CONSTRUCTION WHICH HAVE NOT BEEN BROUGHT TO THE ATTENTION OF THE OWNER. THE MEANS OF CORRECTING ANY ERROR SHALL FIRST BE APPROVED BY THE ARCHITECT AND OWNER.

5 ALL PLAN DIMENSIONS ARE FROM FACE OF MEMBERS UNLESS OTHERWISE INDICATED.

6 ALL CONSTRUCTION SHALL COMPLY WITH THE APPLICABLE BUILDING CODES AND LOCAL RESTRICTIONS. ALL CONTRACTORS MUST COMPLY WITH THE CONTRACTOR REGISTRATION REQUIREMENTS OF ALL GOVERNING AUTHORITIES.

7 IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING MECHANICAL AND ELECTRICAL SERVICES AND DISTRIBUTION SYSTEMS WHETHER SHOWN OR NOT, AND TO PROTECT THEM FROM DAMAGE. THE CONTRACTOR SHALL BEAR ALL EXPENSE OF REPAIR OR REPLACEMENT OF UTILITIES OR OTHER PROPERTY DAMAGED BY OPERATIONS IN CONJUNCTION WITH THE PERFORMANCE OF THE WORK.

8 THE CONTRACTOR IS RESPONSIBLE FOR RECEIVING, UNLOADING, UNCRATING, AND INSTALLATION OF ALL OWNER FURNISHED ITEMS, INCLUDING DISPOSAL OF CRATING, PACKING ETC.

9 THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REPLACEMENT OR REMEDY OF ANY FAULTY, IMPROPER OR INFERIOR MATERIALS OR WORKMANSHIP WHICH SHALL APPEAR WITHIN ONE (1) YEAR OR AS OTHERWISE SPECIFIED FOR A SPECIFIC COMPONENT AFTER THE COMPLETION AND ACCEPTANCE OF THE WORK UNDER THIS CONTRACT.

10 SILICONE CAULK SHALL BE USED AT THE FOLLOWING LOCATIONS:

1. CONDUIT AND PIPE PENETRATIONS AT WALLS, FLOORS, AND CEILINGS-CLEAR
2. STAINLESS STEEL TO WALLS-CLEAR
3. ALL UNLIKE SURFACES-CLEAR

11 CONTRACTOR SHALL CLEAN WORK AREAS ON A DAILY BASIS SO AS NOT TO ACCUMULATE DEBRIS.

12 ALL DEBRIS SHALL BE REMOVED FROM PREMISES AND AREA OF CONSTRUCTION SHALL BE LEFT IN A "BROOM" CLEAN CONDITION AT ALL TIMES. AREAS NOT UNDER TOTAL CONSTRUCTION SHALL BE THOROUGHLY CLEAN AND FREE OF CONSTRUCTION DUST EACH DAY. UPON PROJECT COMPLETION CONTRACTOR IS TO CLEAN WORK AREAS AND JOB SITE THOROUGHLY SO AS TO REMOVE ALL CONSTRUCTION DUST, RESIDUE, AND DEBRIS.

13 CONTRACTOR SHALL NOT OBSTRUCT STREETS, SIDEWALKS, ALLEYS OR OTHE RIGHT-OF-WAY WITHOUT FIRST OBTAINING PROPER PERMITS.

14 ALL WORK SHALL BE ACCOMPLISHED WITH QUALITY WORKMANSHIP OF THE HIGHEST INDUSTRY STANDARDS. ALL MATERIALS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS AND RECOMMENDATIONS.

15 THE OWNER MAY PERFORM ADDITIONAL WORK THAT IS NOT PART OF THIS CONTRACT WITH HIS OWN FORCES, UNDER SEPARATE CONTRACTS AND/OR WITH OTHER CONTRACTORS. THE CONTRACTOR SHALL COOPERATE WITH THE OWNER AND OTHER CONTRACTORS AND COORDINATE HIS WORK WITH THE OWNER SO THAT WORK BY OTHERS CAN BE INCORPORATED IN A TIMELY MANNER.

16 THE CONTRACTOR SHALL BE RESPONSIBLE FOR JOB SAFETY, AND SHALL TAKE ALL NECESSARY PRECAUTIONS TO ENSURE SAFETY OF WORKERS AND OCCUPANTS AT ALL TIMES.

17 ALL ELECTRICAL, MECHANICAL, AND PLUMBING WORK SHALL CONFORM TO STATE AND LOCAL REQUIREMENTS.

18 REMODEL WORK SHALL NOT OBSTRUCT, OR CAUSE TO BE INOPERATIVE TO EXISTING FIRE PROTECTION SYSTEMS. MODIFICATION TO FIRE PROTECTION SYSTEMS SHALL BE PERFORMED BY A FIRE PROTECTION CONTRACTOR, WHO SHALL OBTAIN A PERMIT FROM FIRE LOSS MANAGEMENT PRIOR TO WORK.

19 BIDDING AND CONSTRUCTION IS ADMINISTERED BY THE REMODEL DEPARTMENT OF DARDEN RESTAURANTS INC. THE ARCHITECT SHALL NOT BE CONTACTED DIRECTLY WITHOUT PRIOR APPROVAL OF DARDEN REMODEL DEPARTMENT.

20 THE CONTRACTOR SHALL FURNISH AND INSTALL ALL MINOR ITEMS WHICH ARE OBVIOUSLY AND REASONABLE NECESSARY TO COMPLETE ANY INSTALLATION.

21 DECORATIVE MATERIALS, DRAPES, HANGINGS, ETC., SHALL BE NON FLAMMABLE OR FLAME-PROOF PER STATE FIRE MARSHALL REQUIREMENTS.

22 NOT USED.

23 ACCEPT PREMISES AS FOUND. OWNER ASSUMES NO RESPONSIBILITY FOR THE CONDITION OF THE EXISTING BUILDING SITE OR EXISTING STRUCTURES AT THE TIME OF BIDDING OR THERE AFTER.

24 NOT USED.

25 THE CONTRACTOR SHALL PROVIDE THE RESTAURANT GENERAL MANAGER ONE SET OF "AS BUILT" DRAWINGS.

26 ALL WORK DESCRIBED HEREIN IS CONTRACTOR PERFORMED UNLESS OTHERWISE NOTED.

27 PAINT DAMAGED AREAS OF ANY EXISTING MEMBERS AROUND THE NEW CONSTRUCTION. COORDINATE WITH OWNER.

28 THE GENERAL BUILDING PERMITS SHALL BE PAID FOR BY THE CONTRACTOR, ALL OTHER PERMITS SHALL BE SECURED & PAID FOR BY THE SUBCONTRACTOR DIRECTLY RESPONSIBLE. ALL REQUIRED CITY, COUNTY AND/OR STATE LICENSES SHALL BE ACQUIRED AND PAID FOR BY THE INDIVIDUAL GENERAL CONTRACTOR OR SUBCONTRACTOR.

29 NOT USED.

30 CONTRACTOR SHALL COORDINATE ALL SUB-CONTRACTORS THAT ARE UNDER DIRECT CONTRACT WITH DARDEN RESTAURANTS, INC. THIS INCLUDES COORDINATION, SCHEDULING AND MONITORING. THESE CONTRACTORS ARE, BUT ARE NOT LIMITED TO: IBM, ADT, AEI, COKE, PERLICK, AND THE CARPET INSTALLERS.

31 APPROVED PLANS SHALL BE KEPT IN A PLAN BOX AND SHALL NOT BE USED BY WORKMEN. ALL CONSTRUCTION SETS SHALL REFLECT SAME INFORMATION. THE CONTRACTOR SHALL ALSO MAINTAIN, IN GOOD CONDITION, ONE COMPLETE SET OF PLANS WITH ALL REVISIONS, ADDENDUMS AND CHANGE ORDERS, ON THE PREMISES AT ALL TIMES. THESE THE JOB SUPERINTENDENT.

32 THE ARCHITECT, AND/OR OWNER WILL REVIEW SHOP DRAWINGS AND SAMPLES FOR CONFORMANCE WITH DESIGN CONCEPT FOR THE PROJECT. THE OWNER AND/OR ARCHITECT'S APPROVAL OF AN SPECIFIC ITEM SHALL NOT NECESSARILY INDICATE APPROVAL OF AN ASSEMBLY IN WHICH THE ITEM FUNCTIONS. GENERAL CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS PRIOR TO SUBMISSION AND ASSURE THAT THE INFORMATION PROVIDED IS IN CONFORMANCE WITH THE CONSTRUCTION DRAWINGS.

33 NOT USED.

34 GENERAL CONTRACTOR IS TO ASSURE THAT NO REBAR OR REINFORCEMENT IS PRESENT PRIOR TO CORE DRILLING OR PLACING BOLTS OR ANY OTHER ITEM WHICH COULD DISTURB THE STRUCTURAL SLAB.

35 FOR CONSTRUCTION DETAILS NOT SHOWN, USE THE MANUFACTURER'S STANDARD DETAILS OR APPROVED SHOP DRAWINGS/DATA SHEETS IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

36 INSTALLATION OF ALL OWNER SUPPLIED ITEMS IS BY GENERAL CONTRACTOR UNLESS OTHERWISE SPECIFIED.

37 IF, THROUGH ERRORS OR OMISSIONS, THE INTENT TO THE ARCHITECT OR ENGINEER WITH REGARD TO ANY DETAIL IS NOT CLEAR, OR IS CAPABLE OF MORE THAN ONE INTERPRETATION, SUCH MATTERS SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT/ENGINEER IN WRITING BEFORE THE SUBMISSION OF BIDS, AND THE ARCHITECT/ENGINNER SHALL MAKE CORRECTION OR EXPLANATION IN WRITING. OTHERWISE, NOT EXTRA CHARGE WILL BE ALLOWED FOR THE WORK OR MATERIAL WHICH THE ARCHITECT/ENGINEER WILL REQUIRE, PROVIDED THAT IT COMES WITHIN A REASONABLE INTERPRETATION OF THE DRAWINGS AND SPECIFICATIONS.

38 NOT USED.

39 G.C. TO "TIDY UP" AND ORGANIZE CABLES AND WIRES AROUND THE P.O.S. STATIONS AT VISIBLE LOCATIONS, INCLUDING THE SIDE SERVICE ALLEYS.

40 NOT USED.

41 IT IS NOT THE INTENT ON THIS DRAWINGS NOT TO REMOVE ANY STRUCTURAL ELEMENTS. CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY THAT ITEMS SHOWN IN DEMOLITION ARE NOT PART OF THE STRUCTURAL INTEGRITY OF THE BUILDING. CONTRACTOR SHALL NOTIFY THE ARCHITECT PRIOR TO PROCEED WITH ANY WORK.

ABBREVIATIONS

ADDITIONAL ABBREVIATIONS ARE DEFINED IN LEGENDS THROUGHOUT THE DRAWING SET.

AB AIR BARRIER
AB/VR AIR BARRIER/ VAPOR RETARDER
AC ASPHALTIC CONCRETE, AIR CONDITIONING
ACP ACOUSTICAL CEILING PANEL
ACST ACOUSTICAL
AD AREA DRAIN
ADDM ADDENDUM
ADJ ADJUSTABLE
AESS ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
AF ACCESS FLOORING
AFF ABOVE FINISHED FLOOR
AHU AIR HANDLING UNIT
ALT ALTERNATE
ALUM ALUMINUM
ANOD ANODIZED
APPROX APPROXIMATE
ARCH ARCHITECT, ARCHITECTURAL
AUTO AUTOMATIC
AV AUDIO VISUAL
AWP ACOUSTICAL WALL PANEL(S)

BB BULLETIN BOARD
BD BOARD
BLDG BUILDING
BLKG BLOCKING
BO BOTTOM OF
BOT BOTTOM
BRG BEARING
BRZ BRONZE
BSMT BASEMENT
BUR BUILT UP ROOFING

CAB CABINET
CB CATCH BASIN
CEM CEMENT, CEMENTITIOUS
CG CORNER GUARD
CI CAST IRON
CJ CONTROL JOINT
CIP CAST-IN-PLACE
CL CENTERLINE
CLG CEILING
CLO CLOSET
CLR CLEAR
CM CONSTRUCTION MANAGER
CMU CONCRETE MASONRY UNIT
CO CLEAN OUT, CASED OPENING
COL COLUMN
COMM COMMUNICATION
CONC CONCRETE
CONN CONNECTION, CONNECT
CONSTR CONSTRUCTION
CONT CONTINUOUS, CONTINUE
CORR CORRIDOR
CPT CARPET
CSWK CASEWORK
CTR CENTER
CU CUBIC

D DEEP, DEPTH
DBL DOUBLE
DEMO DEMOLISH, DEMOLITION
DET DETAIL
DF DRINKING FOUNTAIN
DIA DIAMETER
DIAG DIAGONAL, DIAGRAM
DIM DIMENSION
DIV DIVIDE, DIVISION
DMPF DAMPPROOF, DAMPPROOFING
DN DOWN
DPM DESIGN PROJECT MANAGER
DR DOOR, DRESSING ROOM
DS DOWNSPOUT
DW DISHWASHER
DWG DRAWING
DWR DRAWER

E EAST
EIFS EXTERIOR INSULATION AND FINISH SYSTEM
EJ EXPANSION JOINT
EL ELEVATION
ELEC ELECTRIC, ELECTRICAL
ELEV ELEVATOR
ENCL ENCLOSE, ENCLOSURE
EOS EDGE OF SLAB
EP ELECTRIC PANEL
EQ EQUAL
EQUIP EQUIPMENT

EW EACH WAY
EXH EXHAUST
EXP EXPANSION, EXPOSED
EXIST EXISTING

FA FIRE ALARM
FBC FLORIDA BUILDING CODE
FCIO FURNISHED BY CONTRACTOR/ INSTALLED BY OWNER
FD FLOOR DRAIN
FDC FIRE DEPARTMENT CIONNECTION
FDTN FOUNDATION
FE FIRE EXTINGUISHER
FEC FIRE EXTINGUISHER CABINET
FH FIRE HYDRANT
FHC FIRE HOSE CABINET
FIG FIGURE
FIN FINISH, FINISHED
FLEX FLEXIBLE
FLR FLOOR, FLOORING
FLUOR FLUORESCENT
FO FACE OF, FINISHED OPENING
FOIC FURNISHED BY OWNER/ INSTALLED BY CONTRACTOR
FP FIRE PROTECTION, FIREPROOF
FR FRAME, FIRE RATED
FRMG FRAMING
FRP FIBERGLASS REINFORCED PLASTIC
FRT FIRE RETARDANT TREATED
FT FOOT, FEET
FTG FOOTING
FUT FUTURE
FVC FIRE HOSE VALVE CABINET
FWC FABRIC WALL COVERING
FWP FABRIC WRAPPED PANEL(S)

GA GAGE
GALV GALVANIZED
GEN GENERAL, GENERATOR
GFRC GLASS-FIBER REINFORCED CONCRETE
GFRG GLASS-FIBER REINFORCED GYPSUM
GFRP GLASS-FIBER REINFORCED PLASTIC
GL GLASS
GYP GYPSUM
GYP BD GYPSUM BOARD

H HIGH
HB HOSE BIBB
HCWD HOLLOW CORE WOOD
HD HAND DRYER
HDBD HARDBOARD
HDW HARDWARE
HDWD HARDWOOD
HM HOLLOW METAL
HO HOLD OPEN
HORIZ HORIZONTAL
HR HOUR
HSS HOLLOW STRUCTURAL STEEL
HT HEIGHT
HVAC HEATING, VENTILATING, AND AIR CONDITIONING

ID INSIDE DIAMETER
IGU INSULATED GLASS UNIT
INCL INCLUDE, INCLUDING
INFO INFORMATION
INSUL INSULATE, INSULATION
INT INTERIOR
IWD INDIRECT WASTE DRAIN

JAN JANITOR
JT JOINT

KIT KITCHEN

L LONG
LAM LAMINATE, LAMINATED
LAU LAUNDRY
LAV LAVATORY
LT LIGHT
LTG LIGHTING

MAS MASONRY
MATL MATERIAL
MAX MAXIMUM
MDF MEDIUM DENSITY FIBERBOARD
MDO MEDIUM DENSITY OVERLAY
MECH MECHANICAL
MED MEDIUM
MEMB MEMBRANE
MEZZ MEZZANINE
MGT MANAGEMENT
MFR MANUFACTURE, MANUFACTURE
MH MANHOLE
MIN MINIMUM, MINUTE
MISC MISCELLANEOUS
MO MASONRY OPENING
MP MANAGING PARTNER
MS MOP SINK
MTD MOUNTED
MTL METAL
MVBL MOVABLE
MWP METAL WALL PANELS

N NORTH
NIC NOT IN CONTRACT
NO NUMBER
NOM NOMINAL
NRC NOISE REDUCTION COEFFICIENT
NTS NOT TO SCALE

OC ON CENTER
OH OVERHEAD
OD OUTSIDE DIAMETER
OPNG OPENING
OPP OPPOSITE
ORD OVERFLOW ROOF DRAIN
OTS OPEN TO STRUCTURE ABOVE

PBD PARTICLEBOARD
PRCST PRECAST
PD PLANTER DRAIN
PERP PERPENDICULAR
PLAM PLASTIC LAMINATE
PLAS PLASTER
PLYWD PLYWOOD
PNL PANEL
PL PROPERTY LINE
PR PAIR
PRKG PARKING
PROP PROPERTY
PT PAINT, POINT, PRESSURE TREATED
PTN PARTITION

R RADIUS, RISER
RCP REFLECTED CEILING PLAN
RD ROOF DRAIN, ROAD
RECPT RECEPTACLE
REF REFER TO, REFERENCE
REF REFRIGERATOR
REINF REINFORCED, REINFORCING
REQD REQUIRED
REV REVISED, REVISION
RFG ROOFING
RM ROOM
RO ROUGH OPENING
RT RIGHT

S SOUTH, SPANDREL
SCWD SOLID CORE WOOD
SCHED SCHEDULE
SD STORM DRAIN
SECT SECTION
SG SAFETY GLASS
SGL SINGLE
SHT SHEET
SHTHG SHEATHING
SIM SIMILAR
SPEC SPECIFICATION
SPKLR SPRINKLER
SPKR SPEAKER
SQ SQUARE
SS SERVICE SINK
SST STAINLESS STEEL
STC SOUND TRANSMISSION CLASS
STD STANDARD
STL STEEL
STOR STORAGE
STRUCT STRUCTURAL
SUSP SUSPENDED

T TREAD, TEMPERED
T&G TONGUE AND GROOVE
TC TRAFFIC COATING
TEL TELEPHONE
TEMP TEMPERATURE, TEMPORARY
TER TERRAZZO
THK THICK, THICKNESS
TO TOP OF
TSTAT THERMOSTAT
TV TELEVISION
TYP TYPICAL

UGND UNDERGROUND
UNF UNFINISHED
UON UNLESS OTHERWISE NOTED

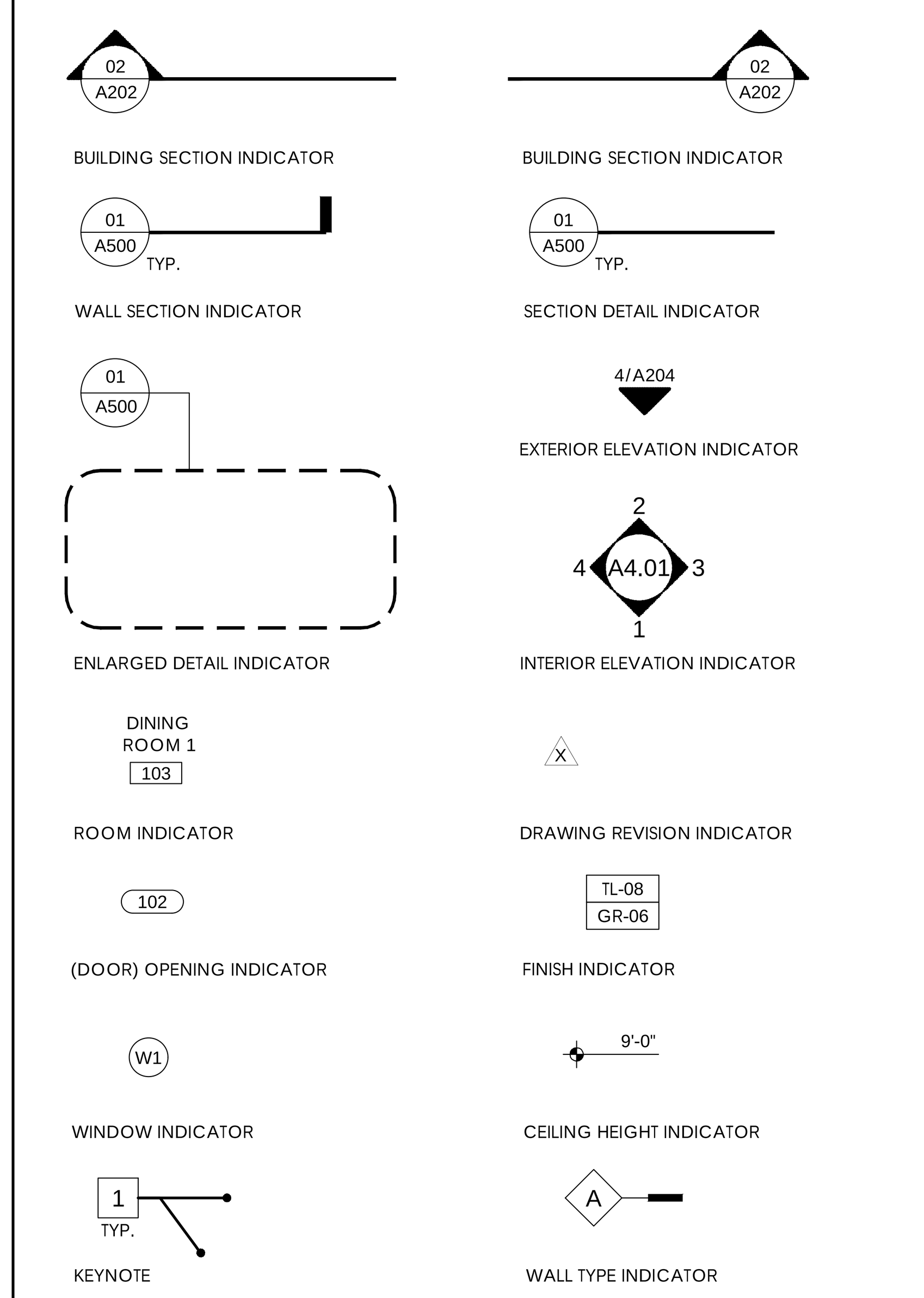
VERT VERTICAL
VEST VESTIBULE
VIF VERIFY IN FIELD
VNR VENEER
VR VAPOR RETARDER
VWC VINYL WALLCOVERING

W WEST, WIDE, WIDTH
W/ WITH
W/O WITHOUT
WC WATER CLOSET, WALLCOVERING
WD WOOD
WH WALL HYDRANT
WP WEATHERPROOF, WATERPROOF, WATERPROOFING, WORK POINT
WR WATER RESISTANT
WT WEIGHT
WWF WELDED WIRE FABRIC

XMFR TRANSFORMER

YD YARD

REFERENCE SYMBOLS



GREASE TRAP CALCULATION:
10 GALLONS PER SEAT WITH TRAP IN SERIES
449 SEATS x 10 = 4490 GALLONS
EXISTING (4) 1250 GALLON TRAPS FOR A TOTAL OF 5000 GALLONS

EXTERIOR SIGNAGE NOTE

ANY AND ALL SIGNAGE DEPICTED WITHIN THESE DRAWINGS ARE SCHEMATIC IN NATURE. ANY APPROVAL RECEIVED FOR THIS BUILDING PERMIT DOES NOT CONSTITUTE APPROVAL FOR THE CONSTRUCTION OF ANY SIGN / SIGN STRUCTURE COVERED WITH ORANGE COUNTY CODE SECTION 31.5. SIGNAGE MUST COMPLY WITH ORANGE COUNTY CODE SECTION 31.5 OR ANY APPLICABLE APPROVED SIGN PLAN. ALL PROPOSED SIGNAGE, INCLUDING SIGNAGE STRUCTURES, ARE REQUIRED TO SUBMIT A SEPARATE APPLICATION AND SHALL BE PROCESSED FOR REVIEW BY ZONING BUILDING AND DEVELOPMENT ENGINEERING.

PROJECT CONTACTS

OWNER: DARDEN RESTAURANTS, INC.
1000 DARDEN CENTER DRIVE
ORLANDO, FL 32837
CONTACT: FRANK CARRERA
DESIGN & DEVELOPMENT MANAGER
(407)245-4061
FCarrera@darden.com

ARCHITECT: HMD GROUP ARCHITECTS PA
10556 N.W. 26TH STREET
SUITE D-101
DORAL, FL 33172
CONTACT: WILFREDO F. PEREZ
(305) 594-2975
hmdgroup@bellsouth.net

MEP ENGINEER: DEVITA & ASSOCIATES, INC.
1150 EAST WASHINGTON STREET
GREENVILLE, SC 29602
CONTACT: RYAN GRAY
(864) 232-6642
rgray@devitainc.com

STRUCTURAL ENGINEER: SIERRA ENGINEERING GROUP
5825 SUNSET DT SUITE 200
MIAMI, FL 33143
CONTACT: JESUS SIERRA
(305) 665-5445
jsierra@sirraeng.com

SCOPE OF WORK

EXISTING PATIO:
PROVIDE NEW ROOF ENCLOSURE STRUCTURE AND NEW PATIO FENCING AT LOWER SECTION OF EXISTING WALKWAY WITH NEW PICNIC TABLES.

APPLICABLE CODES

BUILDING CODE: 2020 FLORIDA BUILDING CODE
ACCESSIBILITY CODE: 2020 FLORIDA BUILDING CODE CHAPTER - 11 ACCESSIBILITY
ELECTRICAL CODE: 2020 NATIONAL ELECTRIC CODE
FIRE CODE: 2020 FLORIDA FIRE PREVENTION CODE

PROJECT INFORMATION

PROJECT USE: RESTAURANT
OCCUPANCY GROUP: A-2 (ASSEMBLY)

NEW ROOF AREA AT EXISTING PATIO: 498.OO S.F.

SEATING CAPACITY:	EXISTING	PROPOSED
PATIO AREA UPPER LEVEL:	64	86
PATIO AREA NEW LOWER LEVEL:	-	46
INTERIOR AREAS:		
DINING SEATING:	273	273
BAR STOOL:	44	44
INTERIOR TOTAL:	317	317
GRAND TOTAL:	381	449

BUILDING HEIGHT: (BUILDING HEIGHT WILL NOT BE MODIFIED BY THIS PROJECT) 2

BUILDING IS EQUIPPED WITH AUTOMATIC FIRE SPRINKLER SYSTEM AND FIRE ALARM SYSTEM.

ONCE THE CONTRACTOR IS AWARDED THEY SHALL PROVIDE A COMPLETED WASTE MANAGEMENT PLAN OR SUBMIT A LETTER OF INTENT TO HIRE AN APPROVED WASTE MANAGEMENT COMPANY TO COLLECT AND DOCUMENT ALL CONSTRUCTION WASTE.

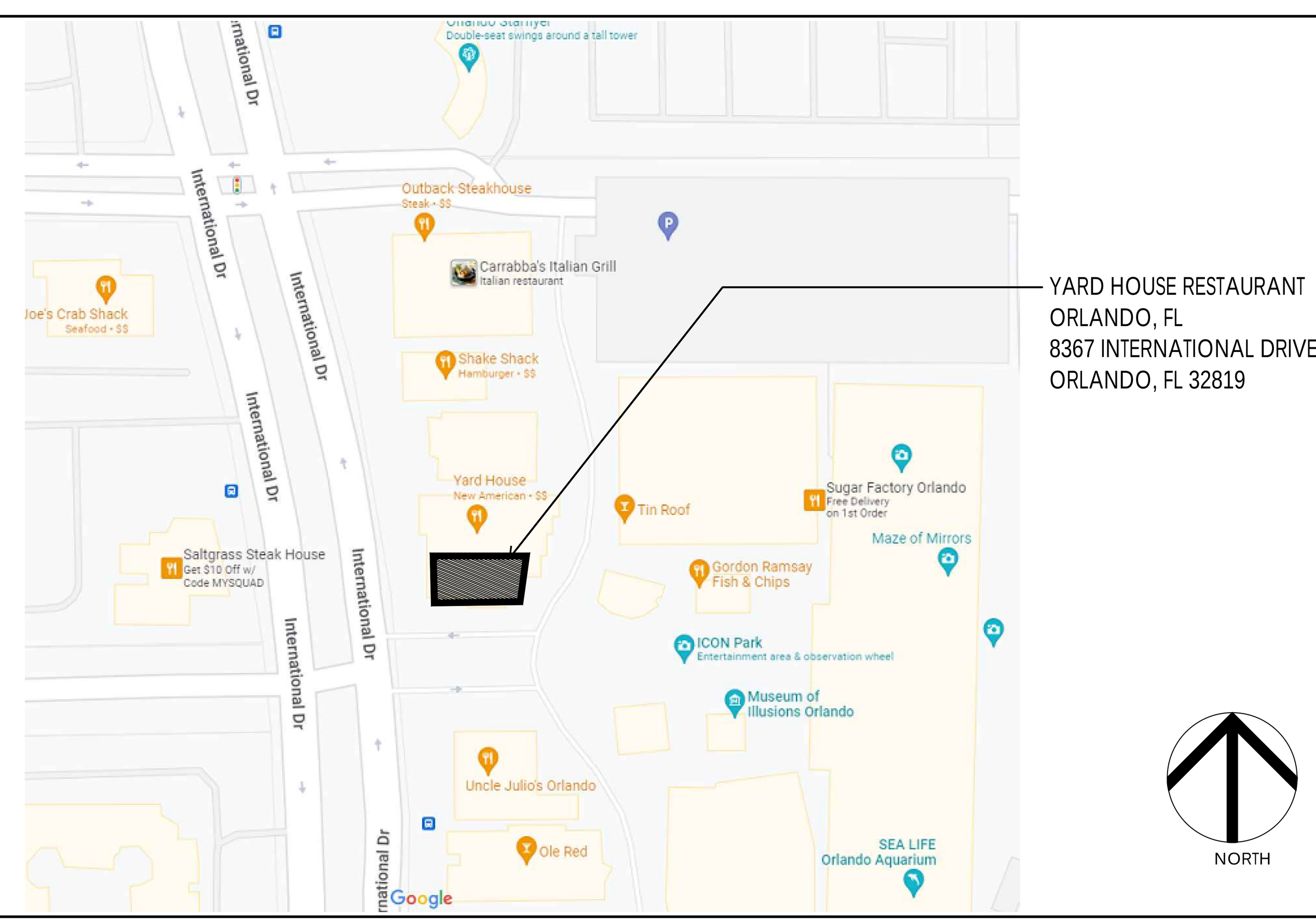
EXISTING PLUMBING FIXTURE COUNTS

EXISTING FIXTURE COUNT BELOW ALLOWS FOR 600 SEATS

FIXTURE TYPE	MEN	WOMEN	FAMILY RESTROOM	EMPLOYEE REST.
	EXISTING	EXISTING	EXISTING	EXISTING
WATER CLOSET	3	4	1	2
LAVATORY	2	2	1	2
URINAL	2			

VICINITY MAP

1



INDEX OF DRAWINGS

● ISSUED ○ REFERENCE ONLY

		DATE / DESCRIPTION: 11-14-2023 BLDG. DEPT. COMMENTS	11-27-2023 BLDG. DEPT. COMMENTS
GENERAL			
C1.1	TITLE SHEET	●	●
D1.1	DEMOLITION PLAN	●	
LS.1	LIFE SAFETY PLAN	●	
A1.0	OVERALL FURNITURE PLAN	●	●
A1.1	FLOOR PLAN	●	
A1.2	REFLECTED CEILING PLAN		
A1.3	ROOF PLAN		
A1.4	EXTERIOR ELEVATIONS		
A1.5	SECTION AND DETAILS		
STRUCTURAL			
S0.0	STRUCTURAL SPECIFICATIONS & GENERAL NOTES		
S0.1	CONCRETE TYPICAL DETAILS		
S1.0	CANOPY FOUNDATION FLOOR PLAN		
S2.0	CANOPY ROOF PLAN		
S3.0	DETAILS		
S4.0	MAIN ROOF DETAILS		
ELECTRICAL			
E1.1	PATIO ELECTRICAL PLAN		
PLUMBING			
P1.1	PLUMBING ROOF DRAINAGE PLAN		
FIRE PROTECTION			
FP1.1	FIRE PROTECTION CRITERIA PLAN		

hmd GROUP PA architects
10556 n.w. 26 street, suite d-101 doral, fl 33172
phone 305.594.2975 fax 305.594.2979 email hmdgroup@bellsouth.net
www: hmdgrouparchitects.com
WILFREDO F. PEREZ
FLORIDA LIC. NO. 13418
LICENSE # AA F000074
ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING

ARCHITECTS PROJECT #: 22-0094

Yard House

Issue Date: 08-05-2022

REVISION INFORMATION

1	BLDG. DEPT. COMMENTS	11-14-2023
2	BLDG. DEPT. COMMENTS	11-27-2023
3		
4		
5		
6		
7		
8		

Restaurant #: 8352

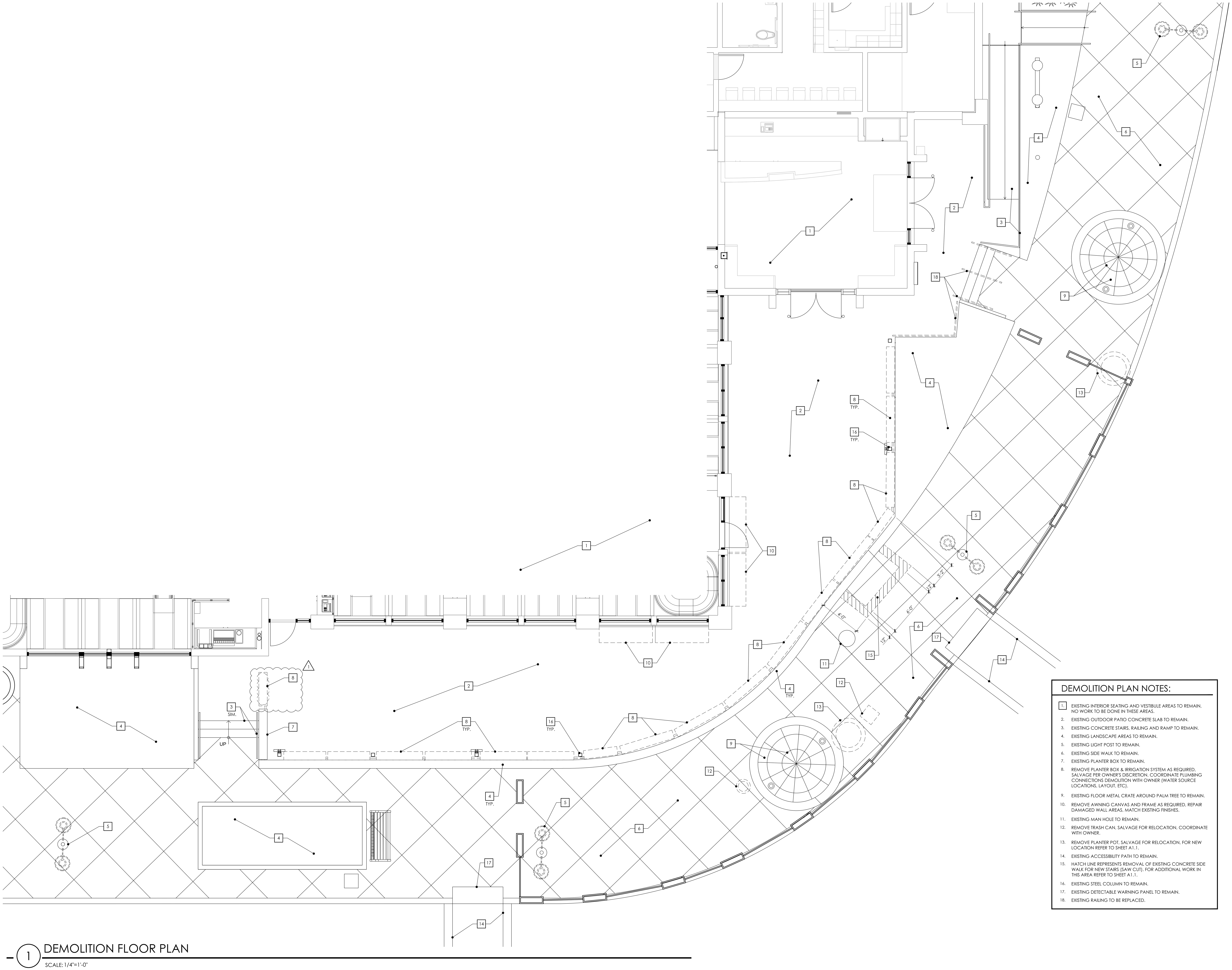
8367 International Drive

Orlando, FL

Drawing

TITLE SHEET

C1.1



1 DEMOLITION FLOOR PLAN

SCALE: 1/4"=1'-0"

DEMOLITION PLAN NOTES:

1. EXISTING INTERIOR SEATING AND VESTIBULE AREAS TO REMAIN, NO WORK TO BE DONE IN THESE AREAS.
2. EXISTING OUTDOOR PATIO CONCRETE SLAB TO REMAIN.
3. EXISTING CONCRETE STAIRS, RAILING AND RAMP TO REMAIN.
4. EXISTING LANDSCAPE AREAS TO REMAIN.
5. EXISTING LIGHT POST TO REMAIN.
6. EXISTING SIDE WALK TO REMAIN.
7. EXISTING PLANTER BOX TO REMAIN.
8. REMOVE PLANTER BOX & IRRIGATION SYSTEM AS REQUIRED, SALVAGE PER OWNER'S DISCRETION. COORDINATE PLUMBING CONNECTIONS DEMOLITION WITH OWNER (WATER SOURCE LOCATIONS, LAYOUT, ETC).
9. EXISTING FLOOR METAL CRATE AROUND PALM TREE TO REMAIN.
10. REMOVE AWNING CANVAS AND FRAME AS REQUIRED, REPAIR DAMAGED WALL AREAS, MATCH EXISTING FINISHES.
11. EXISTING MAN HOLE TO REMAIN.
12. REMOVE TRASH CAN, SALVAGE FOR RELOCATION, COORDINATE WITH OWNER.
13. REMOVE PLANTER POT, SALVAGE FOR RELOCATION, FOR NEW LOCATION REFER TO SHEET A1.1.
14. EXISTING ACCESSIBILITY PATH TO REMAIN.
15. HATCH LINE REPRESENTS REMOVAL OF EXISTING CONCRETE SIDE WALK FOR NEW STAIRS (SAW CUT). FOR ADDITIONAL WORK IN THIS AREA REFER TO SHEET A1.1.
16. EXISTING STEEL COLUMN TO REMAIN.
17. EXISTING DETECTABLE WARNING PANEL TO REMAIN.
18. EXISTING RAILING TO BE REPLACED.

hmd GROUP PA architects
10556 n.w. 26 street, suite d-101 doral, fl 33172
phone 305.594.2975 fax 305.594.2979 email hmdgroup@bellsouth.net
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ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING

ARCHITECTS PROJECT #: 22-0094

Yard House

Issue Date:	08-05-2022
REVISION INFORMATION	
1 BLDG. DEPT. COMMENTS	11-14-2023
2	
3	
4	
5	
6	
7	
8	
Restaurant #:	8352

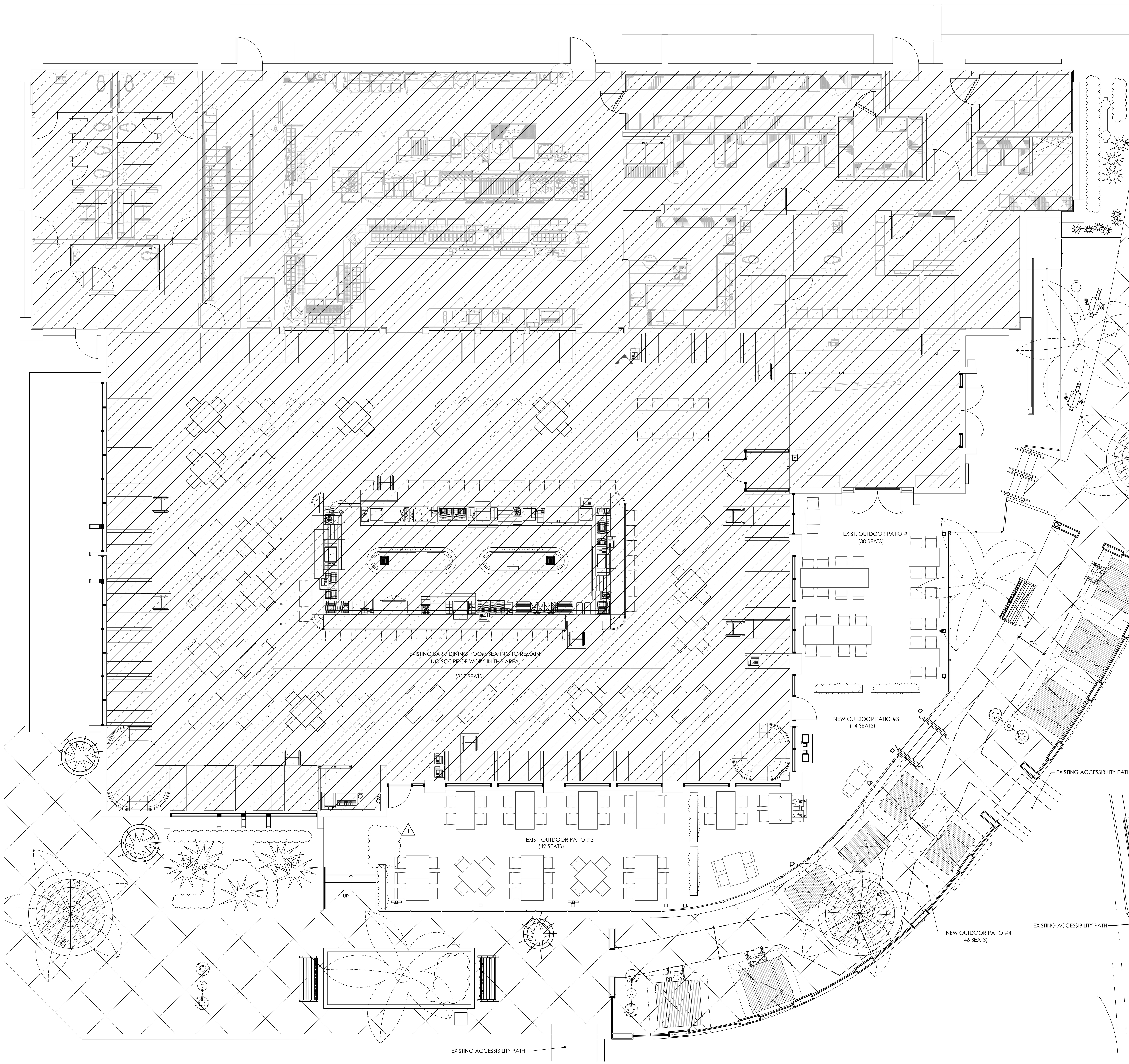
8367 International Drive

Orlando, FL

Drawing

DEMOLITION PLAN

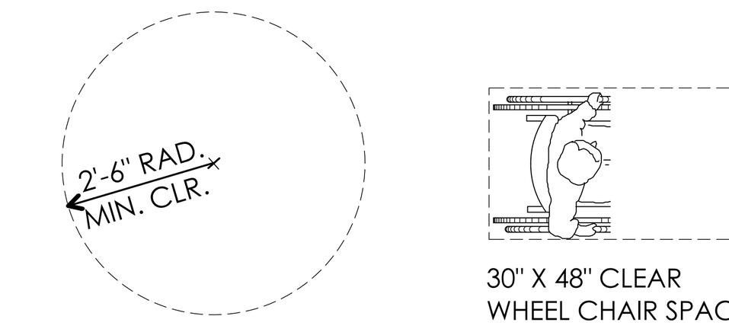
D1.1

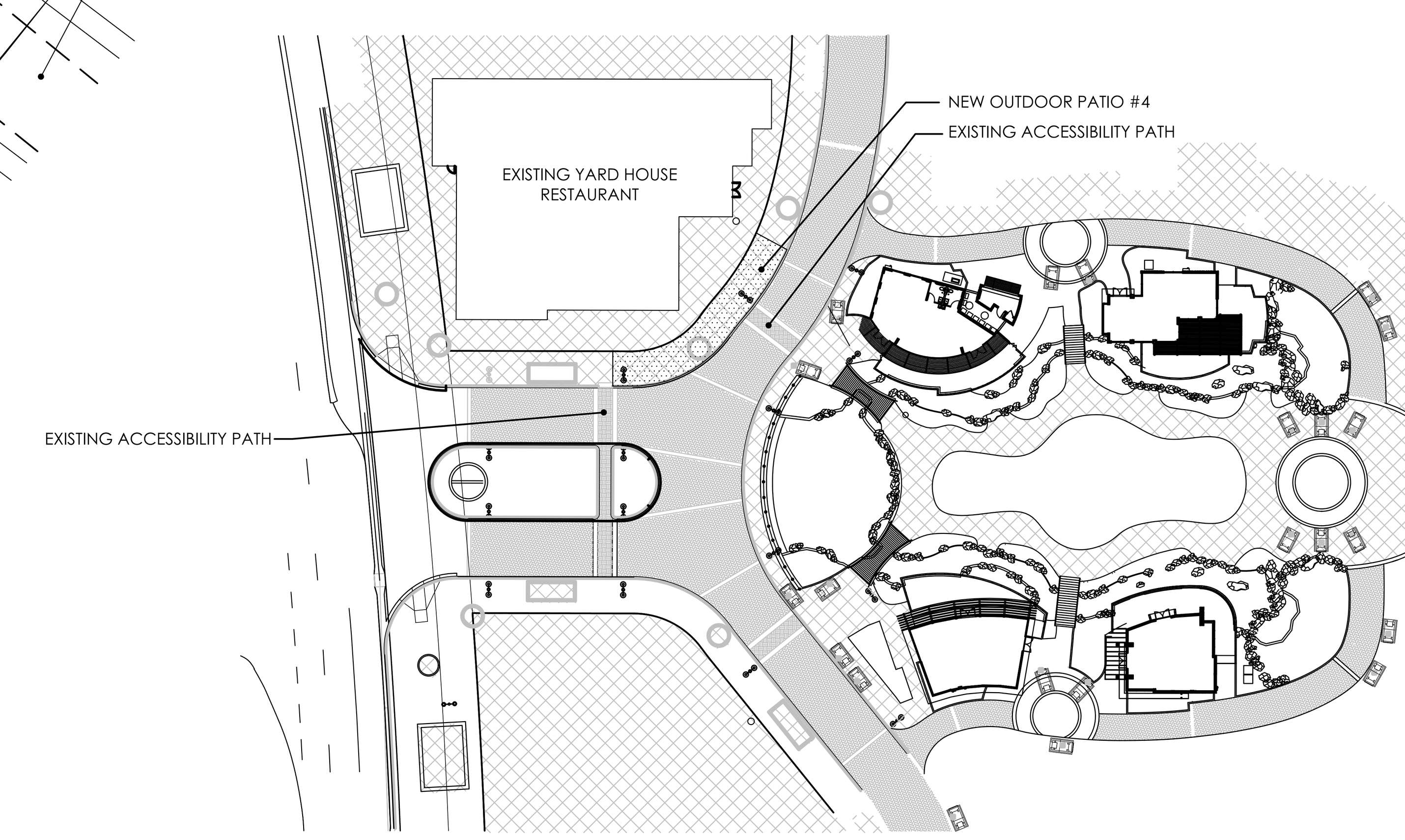


1 LIFE SAFETY PLAN

SCALE: 3/16"=1'-0"

ADA LEGEND





A LOCATION PLAN

SCALE: N.T.S.

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architects
10556 n.w. 26 street, suite d-101 doral, fl 33172
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WILFREDO F. PEREZ
FLORIDA LIC. NO. 13418
LICENSE # AA F000074
ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING

ARCHITECTS PROJECT #: 22-0094

Yard House

Issue Date:	08-05-2022
REVISION INFORMATION	
1 OWNER'S CHANGES	11-14-2023
2	
3	
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8	
Restaurant #:	8352

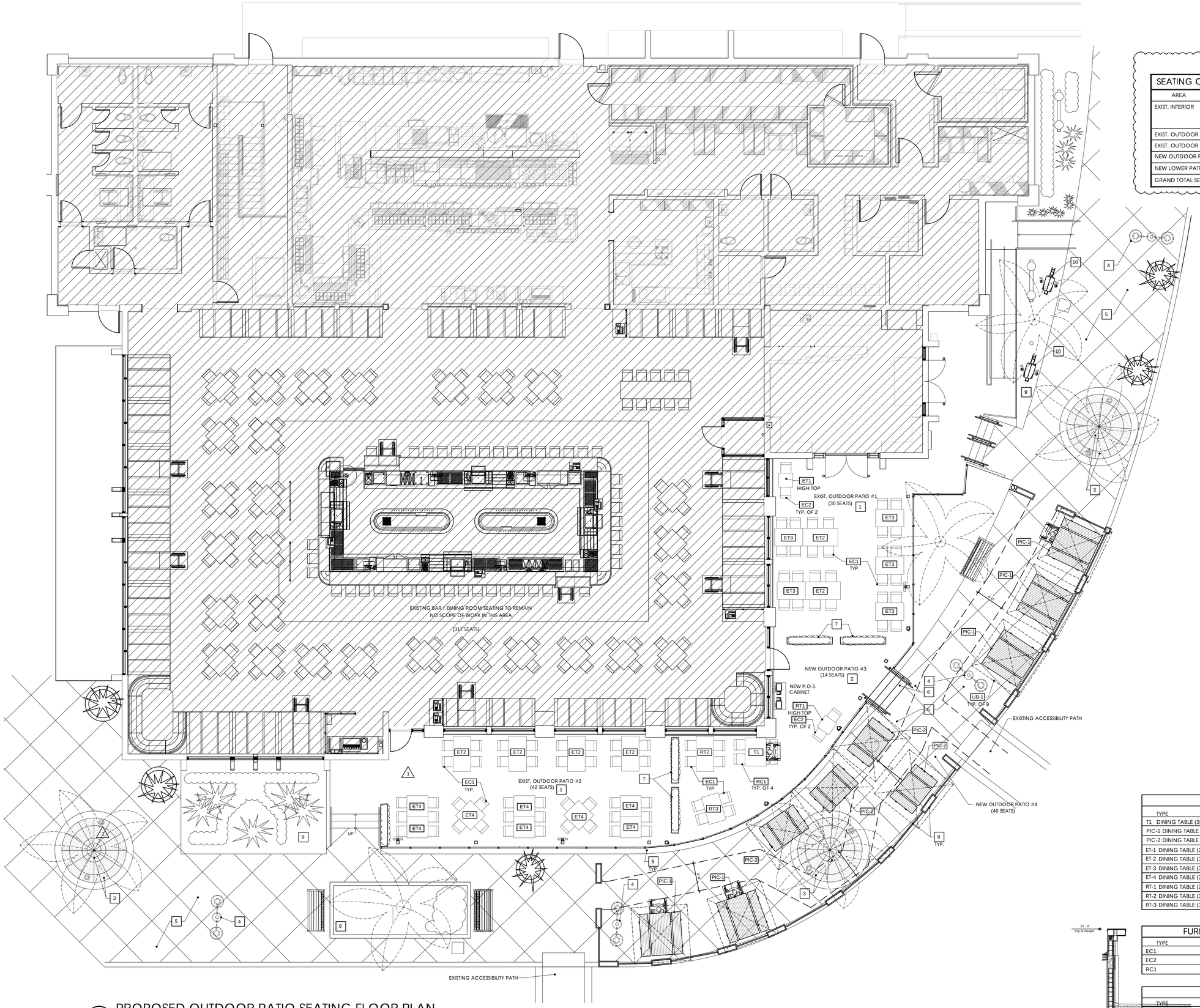
8367 International Drive

Orlando, FL

Drawing

LIFE SAFETY PLAN

LS.1



SEATING CAPACITY SUMMARY

AREA	EXISTING	PROPOSED
EXIST. INTERIOR	DINING SEATING - 273 BAR STOOLS - 44 EXIST. INTERIOR TOTAL - 317	DINING SEATING - 273 BAR STOOLS - 44 INTERIOR TOTAL - 317
EXIST. OUTDOOR COVER PATIO #1	24	30
EXIST. OUTDOOR COVER PATIO #2	40	42
NEW OUTDOOR PATIO #3	-	14
NEW LOWER PATIO #4	-	46
GRAND TOTAL SEATING	381	449

PROPOSED PARTIES

EXIST. OUTDOOR PATIO #1	=	8
EXIST. OUTDOOR PATIO #2	=	9
NEW OUTDOOR PATIO #3	=	4
NEW LOWER PATIO #4	=	9
GRAND TOTAL	=	30

FURNISHING PLAN NOTES:

1. EXISTING OUTDOOR PATIO AREA TO REMAIN.
2. NEW COVERED OUTDOOR PATIO AREA, FOR ADDITIONAL WORK IN THIS AREA, REFER TO SHEET A1.1.
3. EXISTING FLOOR METAL CRATE AND PALM TREE TO REMAIN.
4. EXISTING LIGHT POST TO REMAIN.
5. EXISTING CONCRETE SIDEWALK TO REMAIN.
6. NEW STAIR AND RAILING, REFER TO SHEET A1.1.
7. INSTALL OWNER PROVIDED PLANTER BOX ON CASTERS, COORDINATE WITH OWNER.
8. INSTALLED OWNER PROVIDED FENCING PANELS AND PLANTER BOXES, BY "SELECT SPACE PARTITIONS" COORDINATE WITH OWNER.
9. EXISTING LANDSCAPING AREA TO REMAIN.
10. NEW ALUMINUM KEG MAN ART WORK, REWORK LANSDCAPE AREAS AS REQUIRED.

GENERAL NOTES:

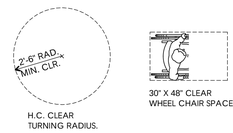
A. PROVIDE SIGN AT LOBBY & PATIO AREA WITH TOTAL SEATING CAPACITY. LOCATION AS PER FIRE DEPARTMENT DISCRETION.

B. NO PATIO TABLES ARE TO BE PERMANENTLY FASTENED TO THE FLOOR. THEY MUST ALL BE MOVEABLE (NO EXCEPTIONS).

C. DIMENSIONS INDICATED AS "MINIMUM CLEAR" ARE CRITICAL AND MUST BE HELD. CONTRACTOR SHALL VERIFY THESE DIMENSIONS PRIOR TO ANY CONSTRUCTION AND SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCY.

D. ALL TABLE TOPS THAT ARE ASSIGNED AS ACCESSIBLE FOR THE HANDICAP SHALL HAVE A 27" MIN. TO BOTTOM OF TABLE TOP, 19" KNEE CLEARANCE AND 34" MAX. TO TOP OF TABLE (COORDINATE WITH OWNER).

ADA LEGEND



FURNITURE SCHEDULE - TABLES

TYPE	DESCRIPTION	QUANTITY	REMARKS
T1 DINING TABLE (36" X 48")	4-TOP TABLES	1	(ADA ACCESSIBLE)
PIC-1 DINING TABLE (70" X 70")	6-TOP TABLES	5	NEW PICNIC TABLE
PIC-2 DINING TABLE (48" X 70")	4-TOP TABLES	4	NEW PICNIC TABLE
ET-1 DINING TABLE (24" X 24")	2-TOP TABLES	1	EXIST. TO REMAIN
ET-2 DINING TABLE (30" X 60")	4-TOP TABLES	6	EXIST. TO REMAIN
ET-3 DINING TABLE (30 X 48")	4-TOP TABLES	5	EXIST. TO REMAIN
ET-4 DINING TABLE (36" X 36")	4 OR 6-TOP TABLES	8	EXIST. TO REMAIN
RT-1 DINING TABLE (24" X 24")	2-TOP TABLES	1	RELOCATED TABLE
RT-2 DINING TABLE (30" X 60")	4-TOP TABLES	1	RELOCATED TABLE
RT-3 DINING TABLE (30" X 48")	4-TOP TABLES	1	RELOCATED TABLE

FURNITURE SCHEDULE - CHAIRS

TYPE	QUANTITY	REMARKS
EC1	78	EXISTING TO REMAIN (LOW CHAIRS)
EC2	4	EXISTING TO REMAIN (HIGH CHAIRS)
RC1	4	NEW RELOCATED CHAIRS (LOW CHAIRS)

UMBRELLA

TYPE	DESCRIPTION	QUANTITY	REMARKS
UB-1	MODEL #RP1225 (8' X 8')	9	BY POGESSI

1 PROPOSED OUTDOOR PATIO SEATING FLOOR PLAN

SCALE: 3/16"=1'-0"

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architects
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ARCHITECTS PROJECT #: 22-0094

Yard House

Issue Date:	08-05-2022
REVISION INFORMATION	
1 BLDG. DEPT. COMMENTS	11-14-2023
2 BLDG. DEPT. COMMENTS	11-27-2023
3	
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8	
Restaurant #:	8352

8367 International Drive

Orlando, FL

Drawing

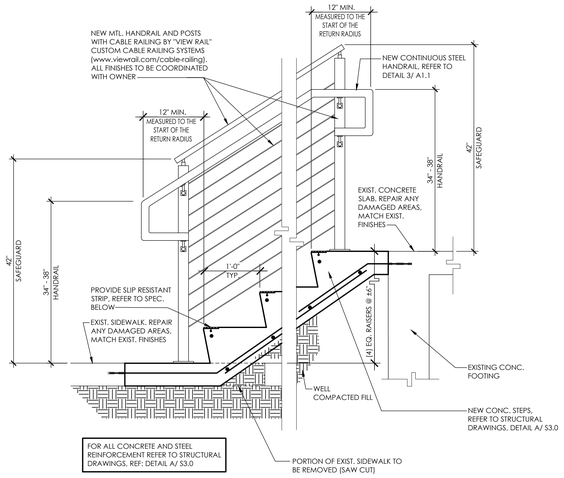
OVERALL FURNITURE PLAN

A1.0

NOTES:

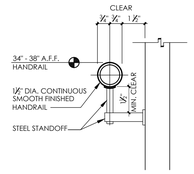
TOP OF GRIPPING SURFACES OF HANDRAILS SHALL BE 34" MIN. AND 38" MAX. VERTICALLY ABOVE WALKING SURFACES, STAIR NOSINGS, AND RAMP SURFACES. HANDRAIL GRIPPING SURFACES SHALL BE CONTINUOUS ALONG THEIR LENGTH AND SHALL NOT BE OBSTRUCTED ALONG THEIR TOPS OR SIDES. THE BOTTOMS OF HANDRAIL GRIPPING SURFACES SHALL NOT BE OBSTRUCTED FOR MORE THAN 20 PERCENT OF THEIR LENGTH. WHERE PROVIDED, HORIZONTAL PROJECTIONS SHALL OCCUR 1½" MINIMUM BELOW THE BOTTOM OF THE HANDRAIL GRIPPING SURFACE. HANDRAILS SHALL BE AT A CONSISTENT HEIGHT ABOVE WALKING SURFACES, STAIR NOSINGS, AND RAMP SURFACES.
AT THE TOP OF A STAIR FLIGHT, HANDRAILS SHALL EXTEND HORIZONTALLY ABOVE THE LANDING FOR 12" MIN. BEGINNING DIRECTLY ABOVE THE FIRST RISER NOSING. EXTENSIONS SHALL RETURN TO A WALL, GUARD, OR THE LANDING SURFACE, OR SHALL BE CONTINUOUS TO THE HANDRAIL OF AN ADJACENT STAIR FLIGHT.
AT THE BOTTOM OF A STAIR FLIGHT, HANDRAILS SHALL EXTEND AT THE SLOPE OF THE STAIR FLIGHT FOR A HORIZONTAL DISTANCE AT LEAST EQUAL TO ONE TREAD DEPTH BEYOND THE LAST RISER NOSING. EXTENSIONS SHALL RETURN TO A WALL, GUARD, OR THE LANDING SURFACE, OR SHALL BE CONTINUOUS TO THE HANDRAIL OF AN ADJACENT STAIR FLIGHT. HANDRAILS SHALL BE DESIGNED & SECURED TO RESIST A LINEAR LOAD OF 50 POUNDS PER LINEAR FOOT AND A CONCENTRATED LOAD OF 200 POUNDS.

CONTRACTOR SHALL PROVIDE SHOP DRAWINGS (SIGNED & SEALED BY A STRUCTURAL ENGINEER).



2 CONCRETE STAIRS DETAIL 1"

PROVIDE **SUPERGRIT NOISING TYPE 231 BF.** PROVIDE FULL LENGTH OF TREAD.



3 HANDRAIL DETAIL 3"

1 FLOOR PLAN

SCALE: 1/4"=1'-0"

LANDSCAPE NOTE

ALL EXISTING EXTERIOR LANDSCAPING AND SHRUBS ON THE FACILITY ARE TO REMAIN AS DEPICTED ON THE APPROVED CDR-23-03-083.

FLOOR PLAN NOTES:

1. EXISTING INTERIOR SEATING OR VESTIBULE AREA TO REMAIN, NO WORK TO BE DONE IN THESE AREAS.
2. EXISTING OUTDOOR PATIO CONCRETE SLAB TO REMAIN.
3. EXISTING CONCRETE STAIRS, RAILING AND RAMP TO REMAIN.
4. EXISTING LANDSCAPE AREAS TO REMAIN.
5. EXISTING LIGHT POST TO REMAIN.
6. EXISTING SIDE WALK TO REMAIN.
7. EXISTING PLANTER BOX TO REMAIN.
8. NEW RAILING, REFER TO SHEET A1.5.
9. EXISTING FLOOR METAL CRATE AND PALM TREE TO REMAIN.
10. EXISTING MAN HOLE TO REMAIN.
11. INSTALL OWNER PROVIDED NEW PLANTER BOX ON CASTERS, COORDINATE WITH OWNER.
12. EXISTING PLANTER POT TO REMAIN.
13. EXISTING STORE FRONT WINDOW TO REMAIN.
14. EXISTING STORE FRONT DOOR OR PAIR DOORS TO REMAIN.
15. EXISTING BUILDING COLUMN TO REMAIN.
16. EXISTING STEEL COLUMN TO REMAIN.
17. NEW PLANTER LIGHT, PROVIDED ELECTRICAL CONNECTION. FOR ADDITIONAL INFORMATION, REFER TO LIGHTING SCHEDULE AT SHEET A1.2.
18. NEW MOBILE P.O.S. UNIT, PROVIDE ELECTRICAL AND DATA CONNECTIONS. REFER TO ELECTRICAL DRAWINGS. COORDINATE WITH OWNER.
19. NEW ALUMINUM KEG MEN ART WORK, PROVIDED BY OWNER INSTALLED BY CONTRACTOR, COORDINATE EXACT LOCATION WITH OWNER, REFER TO DETAIL 3/A1.4. REWORK LANDSCAPE AREAS AS REQUIRED.
20. RELOCATED PLANTER POT, COORDINATE EXACT LOCATION WITH OWNER AND LANDLORD.
21. NOT USED.
22. INSTALL OWNER PROVIDED FREE STANDING FENCING PANELS AND PLANTER BOXES, BY "SELECT SPACE PARTITIONS" COORDINATE WITH OWNER.
23. EXISTING BUILDING WALL FINISHES TO REMAIN.
24. NEW STEEL COLUMN, REFER DETAIL 8/A1.5 AND STRUCTURAL DRAWINGS.
25. EXISTING EDGE OF RAISED PATIO CONCRETE WALL TO REMAIN.
26. EXISTING WAITING BENCH TO REMAIN.
27. NEW CONCRETE STAIR AND RAILING, REFER TO DETAIL 2/A1.1 & STRUCTURAL DRAWINGS.
28. EXISTING STEM PIPE TO REMAIN.
29. LOCATION OF EXISTING HOSE BIB TO REMAIN.
30. NEW ROLLER SHADE COLUMN, PROVIDED AND INSTALLED BY OWNER'S VENDOR, G.C. TO COORDINATE WITH OWNER'S VENDOR THAT OUTER FACE OF NEW ROLLER SHADE COLUMN, ALIGNS WITH INSIDE FACE OF NEW STEEL BEAM ABOVE.
31. EXISTING PEDESTRIAL WALK (FORMALLY A VEHICULAR DRIVE). NO VEHICLES ARE ALLOWED IN THIS AREA.
32. EXISTING DETECTABLE WARNING STRIP AND ACCESSIBLE PATH TO REMAIN.
33. EXISTING 4"x4" METAL DOWNSPOUT DISCHARGING TO EXISTING FLOOR SINK TO REMAIN.
34. EXISTING CONCRETE STAIR TO REMAIN.
35. PROVIDE NEW RAILING, REFER TO DETAIL 2/A1.1 (SIMILAR).

hmd GROUP PA architects
10556 nw 28 street, suite ej 101 doral, fl 33172
phone 305-594-2315 fax 305-594-2979 email hmd@hmdgrouparchitects.com
www.hmdgrouparchitects.com
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INTERIOR DESIGN
CORPORATE PLANNING
WILFREDO F. PEREZ
FLORIDA LIC. NO. 13418
LICENSE # AA 0000774

ARCHITECTS PROJECT #: 22-0094

Yard House

Issue Date:	08-05-2022
REVISION INFORMATION	
1 BLDG. DEPT. COMMENTS	11-14-2023
2	
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8	
Restaurant #:	8352

8367 International Drive

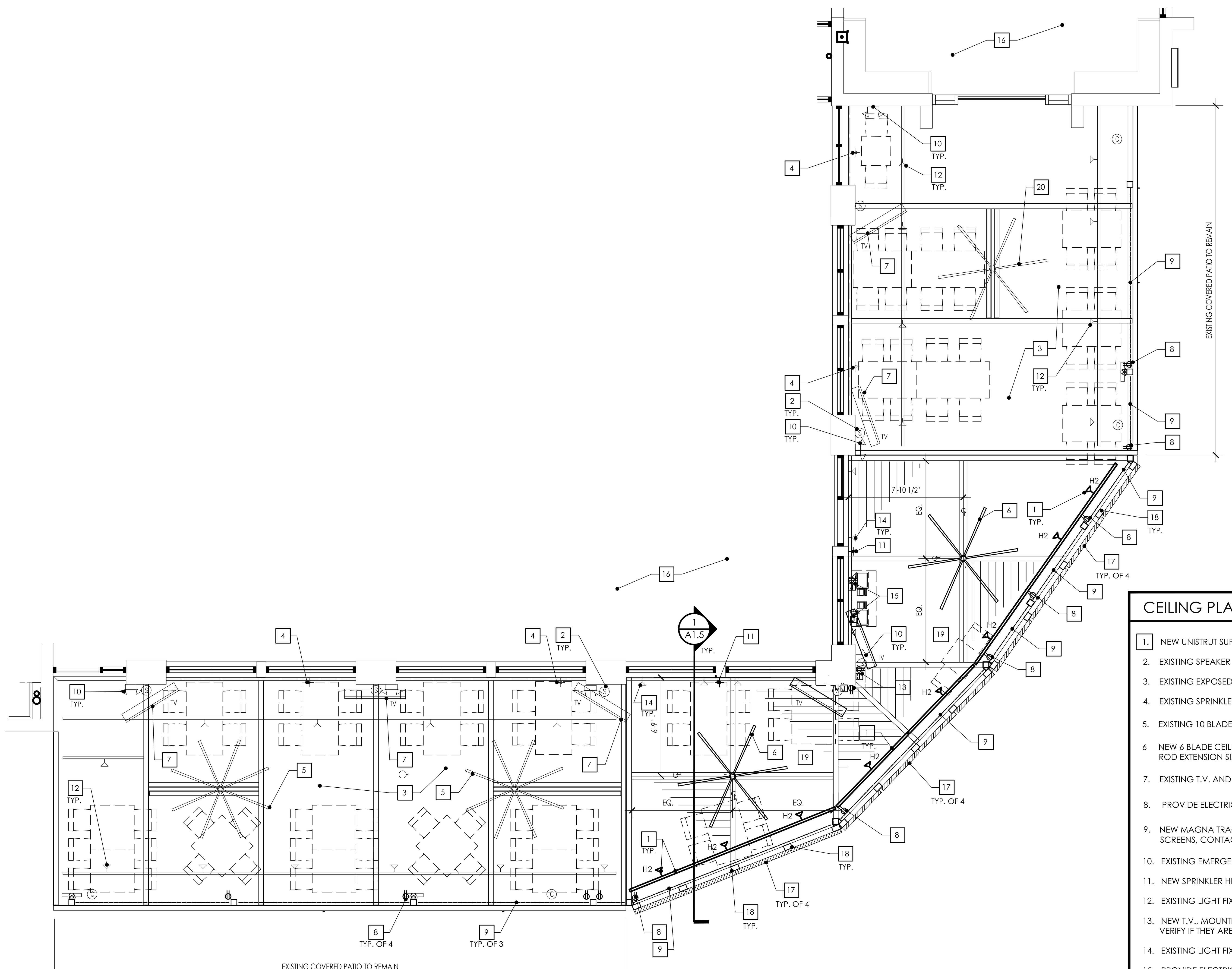
Orlando, FL

Drawing

FLOOR PLAN

A1.1

LIGHTING FIXTURE SCHEDULE 07-19-2022															
TYPE	DESCRIPTION	FINISH	LAMP QTY	LAMP IMAGE	LAMP CODE	DIMMING	VOLTS	WATT.	MOUNTING	MANUFACT.	MODEL NUMBER	FIXTURE SUPPLIED BY	LAMPS SUPPLIED BY	ROOM LOCATION	NOTES
H2	3" SURFACE MOUNTED LED RISE, WET LOCATION, FLOOD LIGHT W/ NARROW SPOT OPTICS 15 DEG., FULL SNOOT ACCESSORY, 10' EXTERNAL CABLE BOTTOM EXIT, AND CANOPY PLATE	BLACK	8	NA	LED, 2700K	ELV	120/277	11.5	SEE PLANS	ECOSENSE	F080-1S-HO-27-8-15-K-F-C / RISE-CANOPY-04-K	VL	NA	EXTERIOR PATIO	PATIO LIGHTS, MOUNTED ON UNISTRUTS
H4B	EXTERIOR LANDSCAPE FLOOD	BLACK	4	NA	-	NA	120	11.5	SURFACE WITH GROUND STAKE	ECOSENSE	F080-1S-HO-BL-8-40-K-H-C / F080-LS-1S-STK-12	-	NA	LANDSCAPE AREA	FOR EXACT LOCATION REFER TO SHEET A1.1



GENERAL NOTES

A. DIMENSIONS AND CONDITIONS TO BE VERIFIED IN FIELD.

B. ALL CEILING HEIGHTS ARE TAKEN FROM FINISH FLOOR.

C. ALL LIGHT FIXTURES SHALL BE SUPPORTED FROM STRUCTURE ABOVE.

D. FOR ELECTRICAL SCOPE OF WORK, REFER TO ENGINEER'S DRAWINGS.

E. FOR LIGHTING FIXTURE SPECIFICATIONS, REFER TO LIGHTING FIXTURE SCHEDULE, THIS SHEET.

F. AUDIO AND SURVEILLANCE SYSTEM TO BE PROVIDED BY OWNER. (IF APPLICABLE)

CEILING PLAN NOTES

1. NEW UNISTRUT SUPPORT SYSTEM WITH SPOT LIGHTS, REFER TO ELECTRICAL DRAWINGS.
2. EXISTING SPEAKER TO REMAIN.
3. EXISTING EXPOSED METAL DECK & STEEL BEAMS TO REMAIN. REPAIR ANY DAMAGED AREAS IF NECESSARY. COORDINATE WITH OWNER.
4. EXISTING SPRINKLER HEAD TO REMAIN. FIELD VERIFY EXISTING LOCATIONS.
5. EXISTING 10 BLADE CEILING FAN TO REMAIN.
6. NEW 6 BLADE CEILING FAN BY BIG ASS 84" FAN, MODEL #MK - 161 - 07 - 18 - 06 - A727 - 124 CONTRACTOR TO VERIFY COLOR, ROD EXTENSION SIZE WITH EXISTING ADJACENT 6 BLADE FAN.
7. EXISTING T.V. AND ELECTRICAL CONNECTIONS TO REMAIN.
8. PROVIDE ELECTRICAL CONNECTION (DUPLEX OUTLET)) FOR OPERABLE ROLLER SHADE, REFER TO ELECTRICAL DRAWINGS.
9. NEW MAGNA TRACK MOTORIZED SCREEN SYSTEM WITH VINYL WINDOWS, INSTALLED AND PROVIDED BY PROGRESSIVE SCREENS, CONTACT: JUSTIN SHOCK- PH. 941 468 3263
10. EXISTING EMERGENCY LIGHT FIXTURE TO REMAIN.
11. NEW SPRINKLER HEAD, CONNECT TO EXISTING SYSTEM AS REQUIRED.
12. EXISTING LIGHT FIXTURE TO REMAIN.
13. NEW T.V., MOUNTED TO MATCH ADJACENT EXISTING T.V. HEIGHT, USE EXISTING ELECTRICAL AND CAT6 CONNECTIONS, VERIFY IF THEY ARE IN GOOD WORKING ORDER. REFER TO ELECTRICAL DRAWINGS.
14. EXISTING LIGHT FIXTURE TO REMAIN, CONTRACTOR TO ASSURE THAT FIXTURE IS IN GOOD WORKING ORDER.
15. PROVIDE ELECTRICAL AND DATA OUTLETS FOR NEW P.O.S., REFER TO ELECTRICAL DRAWINGS.
16. EXISTING INTERIOR SEATING AND VESTIBULE AREAS TO REMAIN, NO WORK TO BE DONE IN THESE AREAS.
17. NEW LOUVER PANEL SECURED TO WALL FACE, REFER TO 7/ A1.5.
18. NEW 4" DEEP x 6" WIDE TUBULAR VERTICAL SUPPORT MEMBER BETWEEN HORIZONTAL BEAMS FOR LOUVER PANELS SUPPORT, REFER TO STRUCTURAL DRAWINGS
19. NEW METAL DECK AND STEEL FRAMING, ALL SURFACES SHALL BE PAINTED BLACK TO MATCH ADJACENT COVERED PATIO SURFACES. COORDINATE WITH OWNER.
20. EXISTING 6 BLADE CEILING FAN TO REMAIN.

1 REFLECTED CEILING PLAN

SCALE: 1/4"=1'-0"

hmd GROUP PA architects

10556 n.w. 26 street, suite d-101 doral, fl 33172

phone 305.594.2975 fax 305.594.2979 email hmdgroup@bellsouth.net

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WILFREDO F. PEREZ
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ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING

ARCHITECTS PROJECT #: 22-0094

Yard House

Issue Date:	08-05-2022
REVISION INFORMATION	
1	
2	
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Restaurant #:	8352

8367 International Drive

Orlando, FL

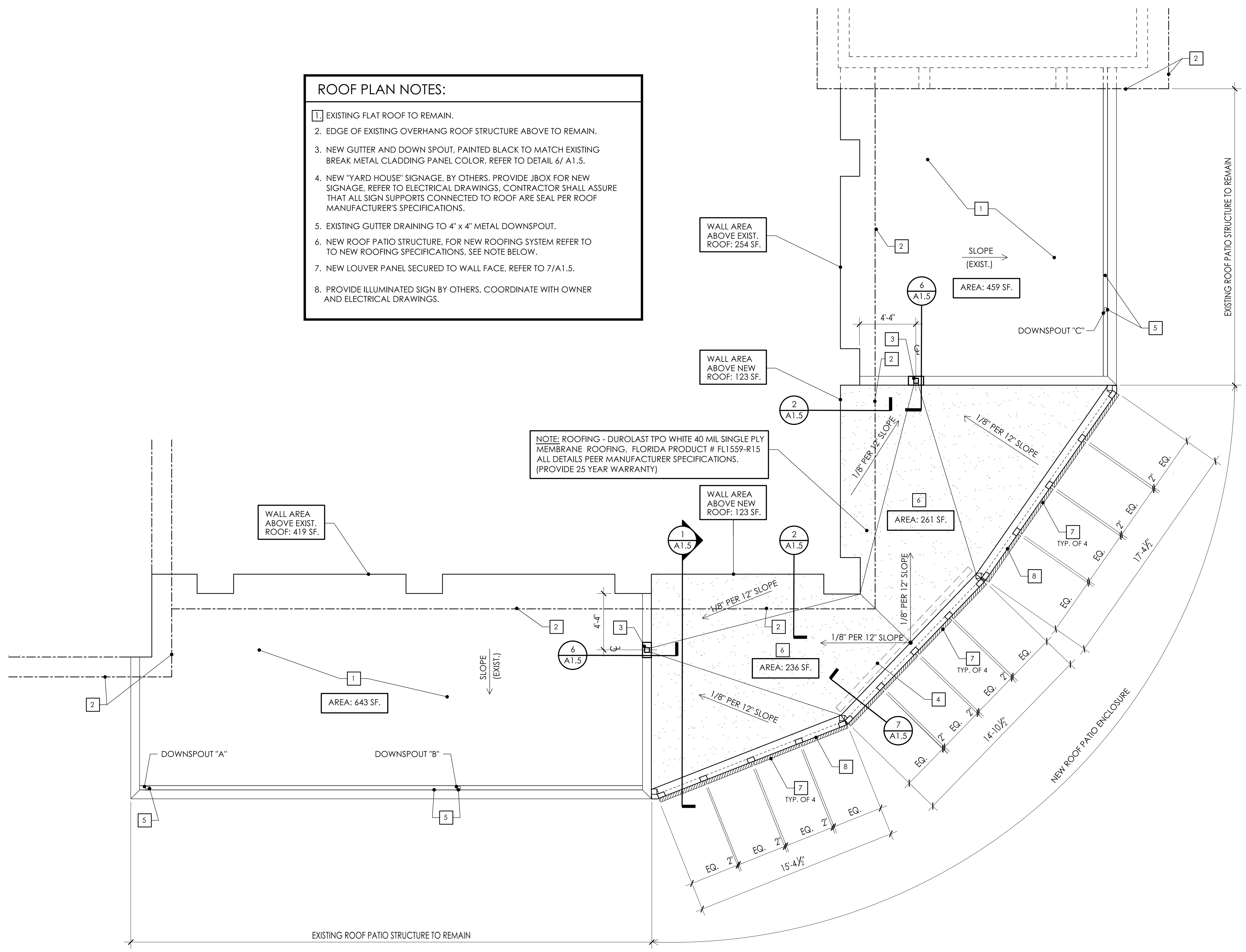
Drawing

REFLECTED CEILING PLAN

A1.2

ROOF PLAN NOTES:

1. EXISTING FLAT ROOF TO REMAIN.
2. EDGE OF EXISTING OVERHANG ROOF STRUCTURE ABOVE TO REMAIN.
3. NEW GUTTER AND DOWN SPOUT, PAINTED BLACK TO MATCH EXISTING BREAK METAL CLADDING PANEL COLOR, REFER TO DETAIL 6/ A1.5.
4. NEW "YARD HOUSE" SIGNAGE, BY OTHERS. PROVIDE JBOX FOR NEW SIGNAGE, REFER TO ELECTRICAL DRAWINGS, CONTRACTOR SHALL ASSURE THAT ALL SIGN SUPPORTS CONNECTED TO ROOF ARE SEAL PER ROOF MANUFACTURER'S SPECIFICATIONS.
5. EXISTING GUTTER DRAINING TO 4" x 4" METAL DOWNSPOUT.
6. NEW ROOF PATIO STRUCTURE, FOR NEW ROOFING SYSTEM REFER TO TO NEW ROOFING SPECIFICATIONS, SEE NOTE BELOW.
7. NEW LOUVER PANEL SECURED TO WALL FACE, REFER TO 7/A1.5.
8. PROVIDE ILLUMINATED SIGN BY OTHERS, COORDINATE WITH OWNER AND ELECTRICAL DRAWINGS.



1 ROOF PLAN

SCALE: 1/4"=1'-0"

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8367 International Drive

Orlando, FL

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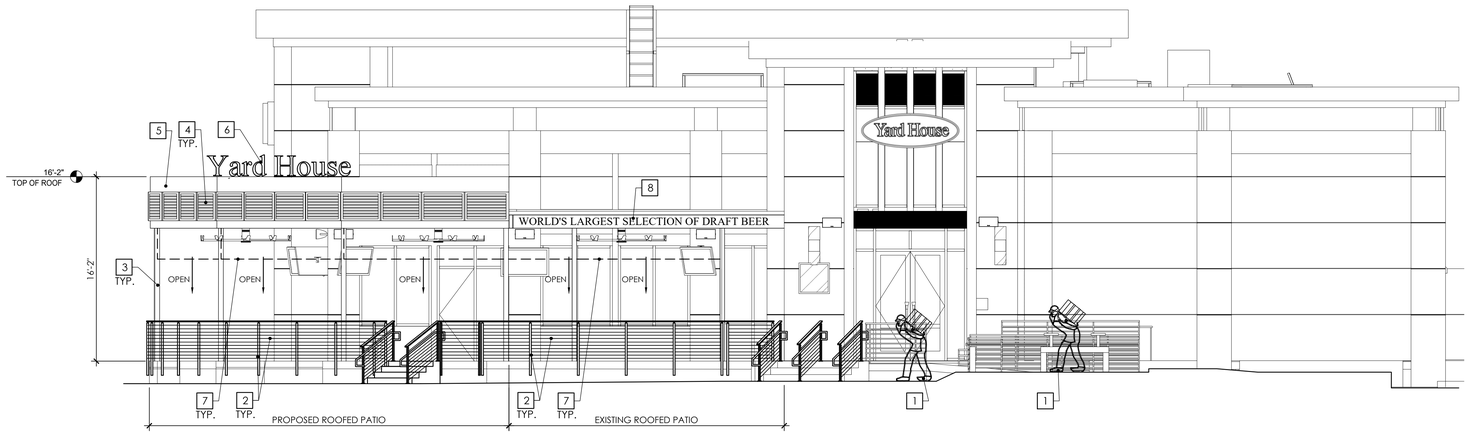
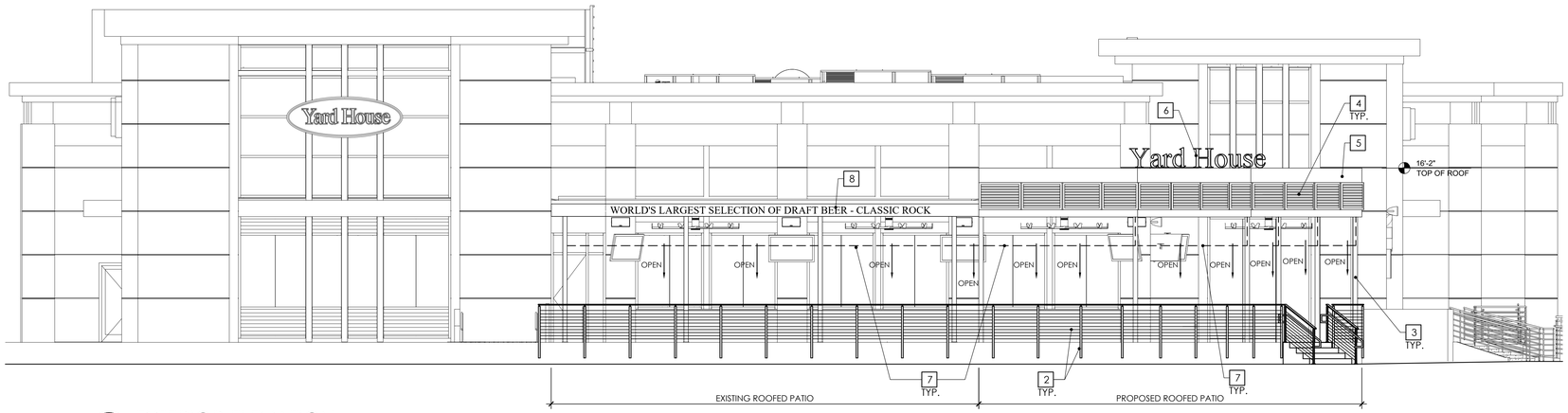
ROOF PLAN

A1.3



1 EXTERIOR ELEVATION

SCALE: 3/16"=1'-0"

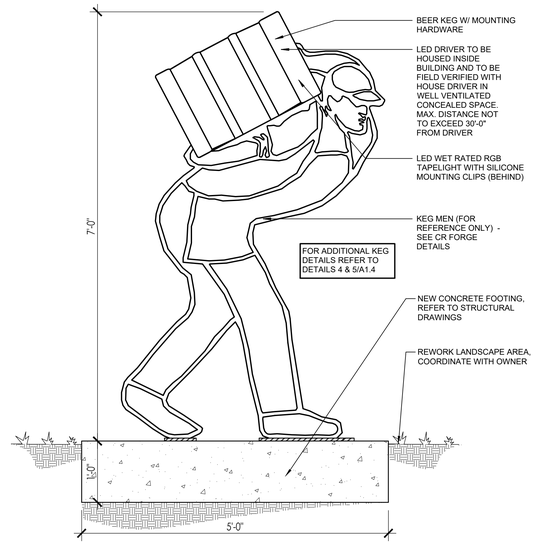


2 EXTERIOR ELEVATION

SCALE: 3/16"=1'-0"

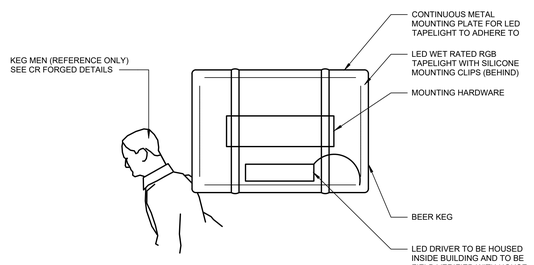
EXTERIOR ELEVATION NOTES:

1. "KEG MEN" FURNISHED AND INSTALLED BY Y.H. VENDOR. G.C. TO PROVIDE BLOCKING AS REQUIRED FOR INSTALLATION. REWORK LANDSCAPES AREAS AS REQUIRED. REPAIR DAMAGED AREAS, MATCH EXISTING. REFER TO DETAILS 3, 4 & 5/ A1.4.
2. NEW RAILING, REFER TO SHEET A1.5.
3. NEW STEEL COLUMN, REFER TO SECTION 1/ A1.5 & STRUCTURAL DRAWINGS.
4. NEW LOUVERED PANELS, REFER TO DETAIL 7/ A1.5.
5. NEW METAL CANOPY, REFER TO SECTION 1/ A1.5.
6. NEW Y.H. ILLUMINATED SIGN BY OTHERS, COORDINATE WITH OWNER AND ELECTRICAL DRAWINGS.
7. NEW ROLLER SHADE. PROVIDE ELECTRICAL CONNECTION. REFER TO DETAIL 7/A1.5 AND ELECTRICAL DRAWINGS. COORDINATE WITH OWNER.
8. PROVIDE NEW ILLUMINATED SIGN, COORDINATE WITH OWNER AND ELECTRICAL DRAWINGS .



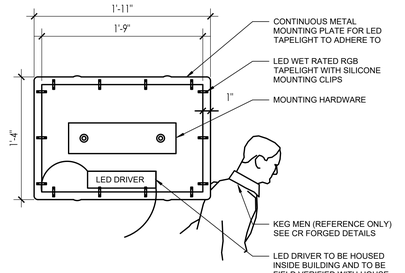
3 KEG MEN - ELEVATION

SCALE: 1"=1'-0"



4 KEG MEN - SECTION

SCALE: 1 1/2"=1'-0"



5 KEG MEN - ELEVATION

SCALE: 1 1/2"=1'-0"

hmd GROUP PA
architects
10556 nw 28 street, suite st 101 doral, fl 33172
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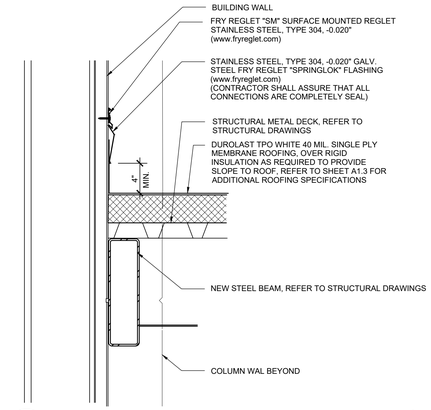
Restaurant #:	8352

8367 International Drive

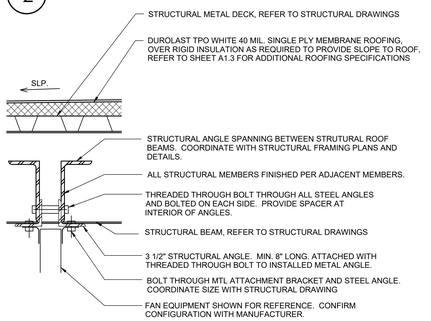
Orlando, FL

Drawing
Exterior Elevations

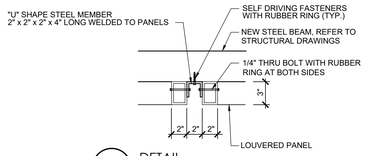
A1.4



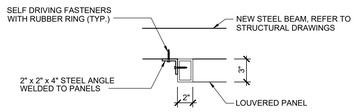
2 DETAIL 1 1/2"



3 DETAIL 1 1/2"



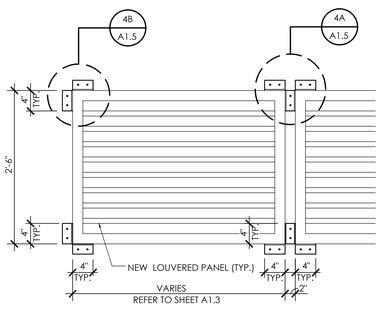
4A DETAIL



4B DETAIL

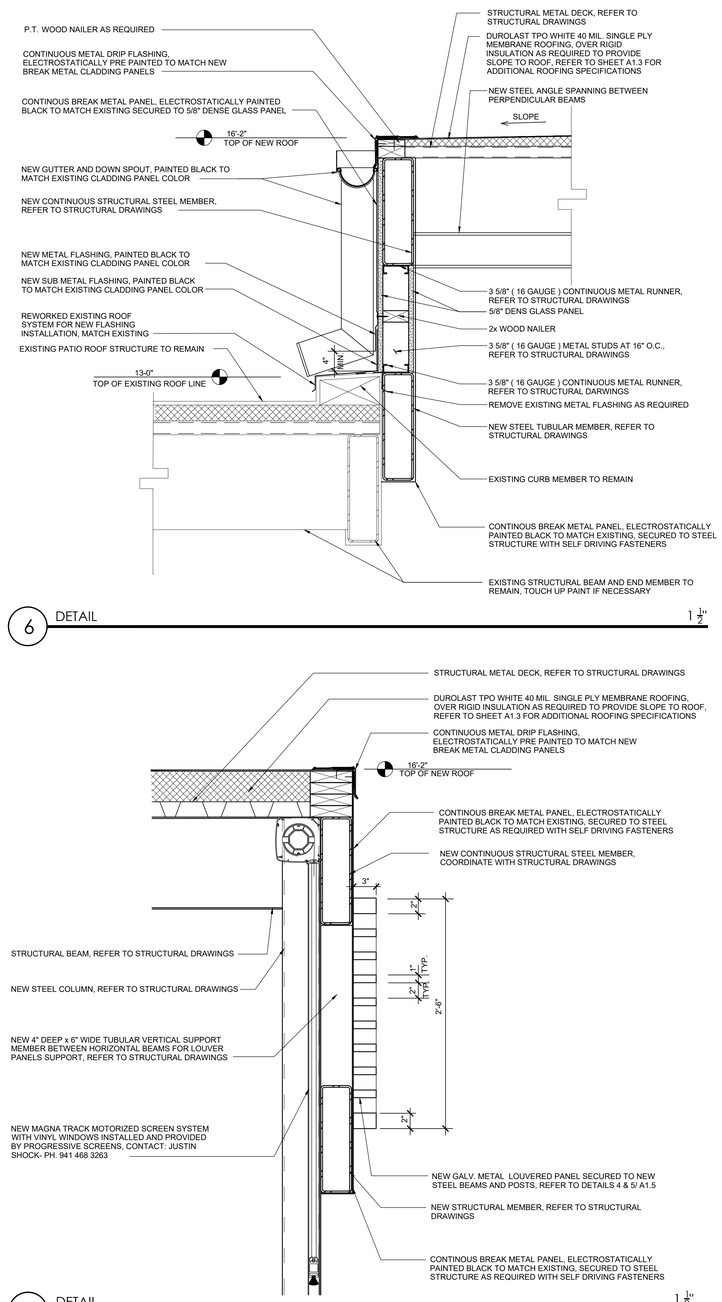
NOTE: ALL MEMBERS TO BE ELECTROSTATICALLY FACTORY PAINTED

4 LOUVERED PANEL DETAIL 1 1/2"



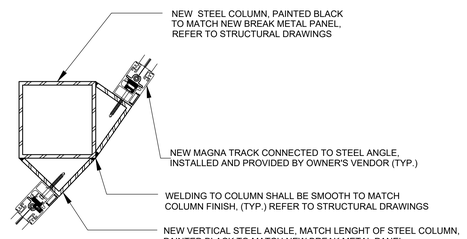
NOTE: ALL MEMBERS TO BE ELECTROSTATICALLY FACTORY PAINTED

5 LOUVER PANEL PARTIAL ELEVATION 1"



6 DETAIL 1 1/2"

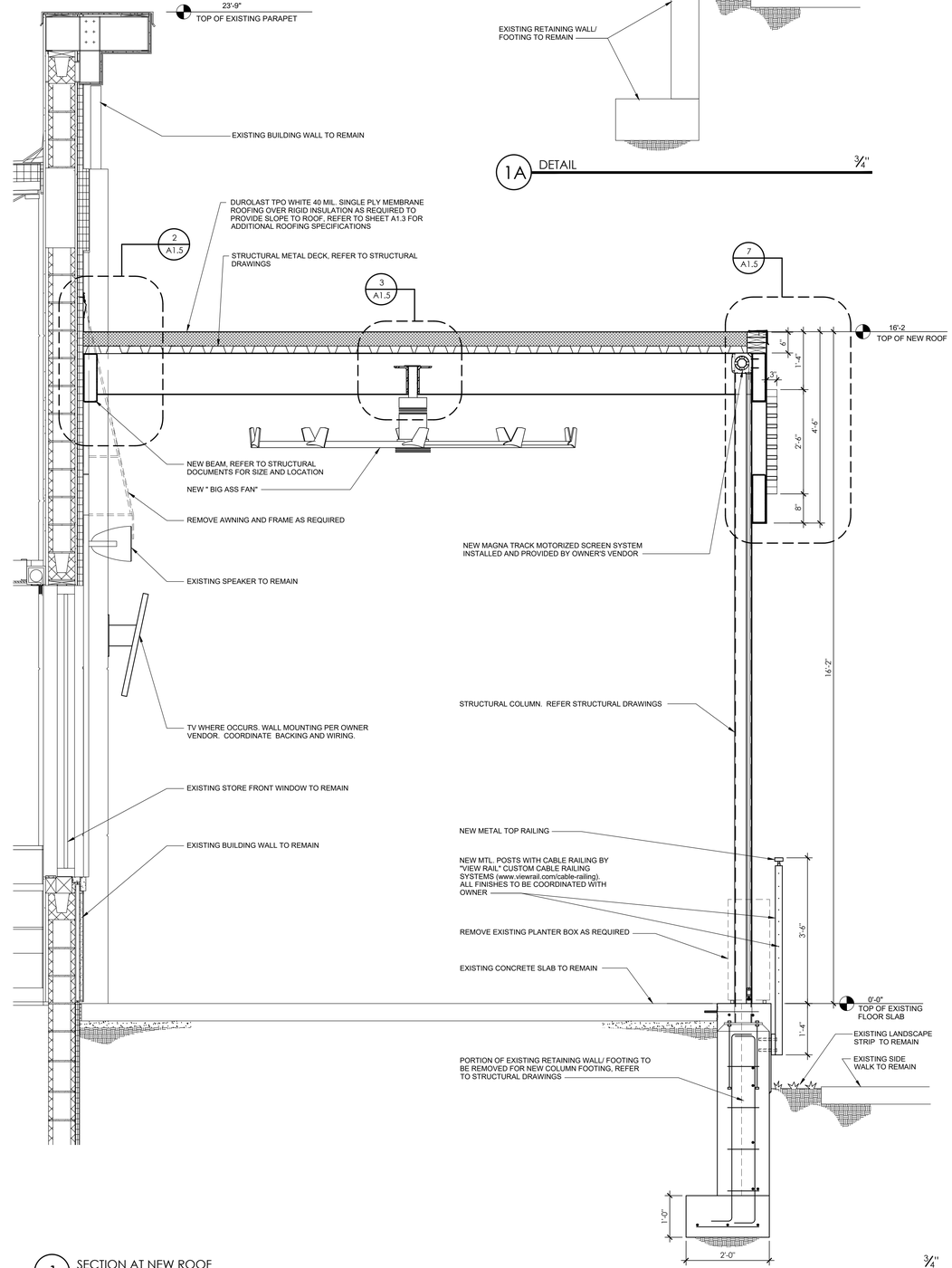
7 DETAIL 1 1/2"



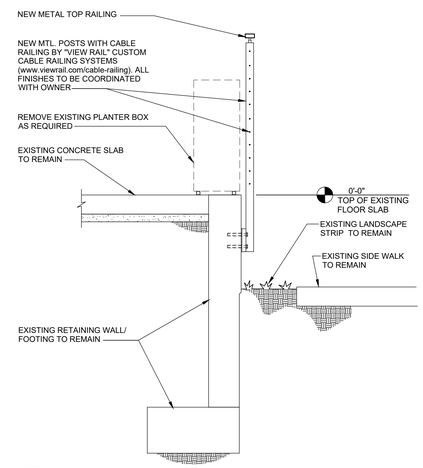
NOTE: ALL MEMBERS TO BE ELECTROSTATICALLY FACTORY PAINTED

8 COLUMN / ROLLER SHADE TRACK DETAIL 3"

CONDITION VARIES, REFER TO SHEET A1.1 AND STRUCTURAL DRAWINGS



1 SECTION AT NEW ROOF 3/4"



1A DETAIL 3/4"

hmd GROUP PA architects
10556 nw 28 street, suite st 101 doral, fl 33172
phone 305-594-2315 fax 305-594-2375 email hmd@hmdgrouparchitects.com
www.hmdgrouparchitects.com
ARCHITECTURE INTERIOR DESIGN CORPORATE PLANNING
WILBERTO F. PEREZ FLORIDA LIC. NO. 13418 LICENSE # AA 0000074

ARCHITECTS PROJECT #: 22-0094

Yard House

Issue Date: 08-05-2022

REVISION INFORMATION

1
2
3
4
5
6
7
8

Restaurant #: 8352

8367 International Drive

Orlando, FL

SECTION & DETAILS

A1.5

MATERIAL DATA

STRUCTURAL SPECIFICATIONS

GENERAL
ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT GOVERNING EDITION OF THE 2020 FLORIDA BUILDING CODE.

PLYWOOD
ALL PLYWOOD SHALL CONFORM TO U.S. PRODUCT STANDARD PS 1-95, AMERICAN PLYWOOD ASSOC. EACH SHEET SHALL BE STAMPED WITH THE PS AND/OR APA GRADEMARK.

ALL PLYWOOD PERMANENTLY EXPOSED TO WEATHER SHALL BE EXTERIOR TYPE PLYWOOD VS. INTERIOR TYPE PLYWOOD AS REFERENCED ABOVE.

CONCRETE
CONCRETE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CURRENT EDITION OF ACI 301 & 318. CONCRETE SHALL BE READY-MIXED CONCRETE IN ACCORDANCE WITH ASTM C94.

MAXIMUM WATER-CEMENT RATIO, BY WEIGHT		
28 DAY COMPRESSIVE STRENGTH	NON-AIR ENTRAINED	AIR ENTRAINED
2500 PSI CONCRETE	.50	.50
3000 PSI CONCRETE	.55	.55

AT THE CONTRACTOR'S OPTION, AN AIR ENTRAINING AGENT CONFORMING TO THE LATEST REVISION OF ASTM SPECIFICATION C260 MAY BE ADDED TO THE CONCRETE TO PROVIDE SPECIFIED AMOUNTS OF ENTRAINED AIR. CEMENT SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR PORTLAND CEMENT PER ASTM DESIGNATION C150, TYPE II.

CONCRETE ELEMENT	MIN 28 DAY COMPRESSIVE STRENGTH	MAX SIZE AGGREGATE (INCHES)	MAX SLUMP (INCHES)	TOTAL AIR CONTENT (%)
SLABS ON GRADE	3000	1	4	4%±1.5%
FOOTINGS	3000	1	4	4%±1.5%

SLUMP WILL BE MEASURED AT THE TRUCK DISCHARGE. PUMPING OF CONCRETE MAY REQUIRE ADMIXTURES TO INCREASE SLUMP BEYOND THE MAXIMUM SLUMP LISTED ABOVE. ADMIXTURES ARE SUBJECT TO THE ENGINEER'S REVIEW. THE SPECIAL INSPECTOR SHALL BE PROVIDED WITH A BATCH TICKET AND WEIGHT TAG UPON DELIVERY OF EACH LOAD OF CONCRETE.

ALL CONCRETE SHALL BE PLACED WITH MECHANICAL VIBRATION UNLESS NOTED OTHERWISE.

EPOXY
EPOXY RESIN ADHESIVE SHALL BE SET-XP AS MANUFACTURED BY SIMPSON STRONG-TIE OR EQUAL(ICC-ES REPORT ESR-2508). THE TYPE AND PROPORTIONS SHALL BE AS RECOMMENDED BY THE MANUFACTURER FOR THE CONDITION AND USE. PREPARATION OF CONCRETE INCLUDING DRILLING OF HOLES FOR ANCHORS, AS WELL AS EPOXY AND ANCHOR INSTALLATION SHALL BE AS RECOMMENDED BY THE MANUFACTURER.

REINFORCING STEEL
BARS FOR REINFORCING SHALL BE GRADE 60 DEFORMED BARS CONFORMING TO ASTM A706 OR A615. LAP SPLICES SHALL BE IN ACCORDANCE WITH ACI 318 - CURRENT EDITION UNLESS NOTED OTHERWISE ON THE PLANS. BARS TO BE WELDED OR FIELD BENT SHALL CONFORM TO ASTM A706.

SLAB MEMBRANE - 6 MIL THICK PLASTIC OR VYNIL MEMBRANE, UNLESS NOTED OTHERWISE.

NON-SHRINK GROUT
NON-SHRINK GROUT SHALL BE FLOWABLE, WITH A MINIMUM 7 DAY COMPRESSIVE STRENGTH OF 5000 PSI. NON-SHRINK GROUT SHALL BE MASTERFLOW 928 GROUT AS MANUFACTURED BY CHEMREX OR APPROVED EQUAL.

MASONRY
MASONRY UNITS SHALL BE LIGHT WEIGHT GRADE N UNITS CONFORMING TO ASTM DESIGNATION C-90 ALL CELLS SHALL BE GROUTED SOLID. f'm = 1500 psi MIN.

MORTAR
MORTAR SHALL CONFORM TO TYPE M AND SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 2500 psi AT 28 DAYS.

GROUT
GROUT SHALL BE COMPOSED OF 1 1/4 PARTS PORTLAND CEMENT, 3 PARTS SAND AND 2 PARTS 3/8" PEA GRAVEL. THE GROUT SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 2500 psi AT 28 DAYS.

LIGHT GAGE METAL FRAMING
1. LIGHT GAGE METAL FRAMING SHALL BE DESIGNED AND FABRICATED BY A FRANCHISED LIGHT GAGE METAL FRAMING FABRICATOR. (ICC-ES ER-4943).
2. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR THE LIGHT GAGE METAL FRAMING TO BE INSTALLED AS SHOWN ON THE STRUCTURAL AND ARCHITECTURAL DRAWINGS, INCLUDING LAYOUT, SIZE OF MEMBERS, AND CONNECTION DETAILS. IN ADDITION TO THE ABOVE DRAWINGS CALCULATIONS SHOWING ALL STRESSES AND DEFLECTION CAUSED BY DEAD AND LIVE LOADS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. DRAWINGS AND CALCULATIONS SHALL BE SIGNED BY A REGISTERED CIVIL ENGINEER OF THE STATE OF CALIFORNIA.
3. LIGHT GAGE METAL FRAMING DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE APPLICABLE REQUIREMENTS OF THE 2007 C.B.C.
4. LIGHT GAGE METAL FRAMING FABRICATOR SHALL OBTAIN ALL NECESSARY APPROVALS FROM THE PUBLIC AGENCIES INVOLVED IN GOVERNING CONSTRUCTION.
5. ARC WELDING ELECTRODES SHALL BE E60 SERIES FOR LIGHT GAUGE METAL FRAMING
6. SECTIONS 18, 20GA, FY = 33 KSI.
7. SECTIONS 16GA, FY = 50 KSI.

FASTENERS
ALL FASTENERS USED FOR LIGHT GAGE METAL ATTACHMENTS TO STRUCTURAL STEEL SHALL BE 1/4" SELF DRILLING SCREWS (S.D.S.) BY HILTI PRODUCT OR EQUAL. ALL FASTENERS USED FOR METAL STUDS AND METAL JOIST CONNECTIONS SHALL BE #10-16 SCREWS U.N.O. (USE FLATHEADS WHERE SURFACE IS TO BE FINISHED WITH GYP. BOARD OR PLYWOOD). ALL FASTENERS USED AT CONCRETE EMBEDMENTS, SHALL BE POWDER DRIVEN FASTENERS BY HILTI PRODUCT OR EQUAL. (ICC-ES REPORT ESR-1663)

STRUCTURAL STEEL AND MISCELLANEOUS IRON
STRUCTURAL STEEL AND MISCELLANEOUS IRON SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CURRENT EDITION OF THE AISC CODE OF STANDARD PRACTICE.

1. WIDE FLANGE AND STRUCTURAL TEE SHAPES SHALL CONFORM TO ASTM A992.
2. CHANNELS AND ANGLES SHALL CONFORM TO ASTM A36.
3. STRUCTURAL PLATE SHALL CONFORM TO ASTM A36 OR ASTM A572 GR50.

ALL STRUCTURAL STEEL AND MISCELLANEOUS IRON SHALL RECEIVE SHOP PRIME COAT EXCEPT ON SURFACES RECEIVING WELDS, EMBEDDED IN CONCRETE, OR AT SLIP CRITICAL HIGH STRENGTH BOLTS WHICH SHALL BE TOUCHED UP AFTER CONNECTION IS COMPLETE. STRUCTURAL STEEL AND MISCELLANEOUS IRON WHICH IS TO HAVE SPRAY ON FIREPROOFING SHALL NOT BE PAINTED. STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL RECEIVE TWO COATS OF SEMI-GLOSS ALKYD ENAMEL COMPATIBLE WITH PRIMER.

HOLLOW STRUCTURAL SECTIONS - ASTM A500, GRADE B

MACHINE BOLTS, ANCHOR BOLTS, STUDS AND THREADED RODS
1. BOLTS AND RODS SHALL CONFORM TO ASTM A307 GRADE A OR B OR A36.
2. NUTS SHALL BE AS SHOWN BELOW AND FINISH SHALL MATCH FASTENER.

FASTENER GRADE AND SIZE	NUT CLASS	NUT STYLE
ASTM A36 OR ASTM A307A, ¼" TO ½"	ASTM A563-A	HEX
ASTM A36 OR ASTM A307A, OVER ½" TO 4"	ASTM A563-A	HEAVY HEX
ASTM A307B, ¼" TO 4"	ASTM A563-A	HEAVY HEX

3. HEADED STUDS AND WELDING SHALL CONFORM TO AWS D1.1-CURRENT EDITION, TYPE B STUDS.

HIGH STRENGTH BOLTS, NUTS, WASHERS & ANCHOR RODS
BOLTS, NUTS, WASHERS AND RODS PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED.
1. BOLTS SHALL BE HEAVY HEX STRUCTURAL BOLTS PER ANSI B18.2.1 AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325. BOLTS SHALL BE TYPE 1,2 OR 3 AND SHALL BE USED IN A BEARING CONNECTION.
2. TENSION CONTROL BOLT ASSEMBLIES SHALL CONFORM TO ASTM F1852.
3. DIRECT-TENSION INDICATORS SHALL CONFORM TO ASTM F959.
4. RODS SHALL CONFORM TO ASTM F1554, GRADE A36.
5. NUTS SHALL BE AS SHOWN BELOW AND FINISH SHALL MATCH FASTENER.

	FASTENER GRADE AND SIZE	NUT CLASS	NUT STYLE
ASTM A325	TYPE 1 AND 2, UNCOATED	ASTM A563-C, C3,D,DH,DH3	HEAVY HEX
	TYPE 1 AND 2, ZINC COATED	ASTM A563-DH	HEAVY HEX
	TYPE 3, UNCOATED	ASTM A563-C3, DH3	HEAVY HEX
ASTM A490	TYPE 1 AND 2, UNCOATED	ASTM A563-DH, DH3	HEAVY HEX
	TYPE 3, UNCOATED	ASTM A563-DH3	HEAVY HEX

6. WASHERS SHALL BE FLAT CIRCULAR, RECTANGULAR OR SQUARE BEVELED WASHERS AND SHALL CONFORM TO ASTM F436 TYPE 1. FINISH SHALL MATCH NUT. WASHERS SHALL BE INSTALLED UNDER THE ELEMENT BEING TURNED FOR A325 BOLTS AND UNDER BOTH THE HEAD AND THE NUT FOR A490 BOLTS. WASHERS OVER OVERSIZED OR SLOTTED HOLES SHALL ALSO COMPLY WITH RCSC SPECIFICATIONS.

METAL DECK
METAL DECK PLN, PLB OR B AND W2 FORMLOK SHALL BE AS MANUFACTURED AND LABELED BY VERCO MANUFACTURING CO. DECK AND ACCESSORIES SHALL BE FORMED OF GALVANIZED STEEL CONFORMING TO CURRENT ASTM CRITERIA. ATTACHMENTS TO COMPLY WITH DIAPHRAGM SHEAR REQUIREMENTS SHOWN ON STRUCTURAL DRAWINGS AND IN ACCORDANCE WITH ICC ES REPORT ESR-2078P.

WELDING
ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS PER AWS "STANDARD QUALIFICATION PROCEDURE" TO PERFORM THE TYPE OF WORK REQUIRED. ALL WELDING SHALL BE IN ACCORDANCE WITH THE CURRENT AWS WELDING CODE. ARC WELDING ELECTRODES SHALL BE E70 SERIES FOR A36, A572 & A992 MATERIAL, AND E80 SERIES FOR A706 REINFORCING STEEL.

WELD METAL TOUGHNESS SHALL BE REPORTED ON THE ELECTRODE MANUFACTURER'S CERTIFICATE OF COMPLIANCE. ALL ELECTRODES SHALL BE LOW HYDROGEN WITH A MINIMUM CVN VALUE OF 20 FT-LBS AT -20° F. EXCEPTIONS: METAL DECK WELDING, STAIR AND HANDRAIL WELDING, LIGHT GAGE STEEL WELDING.

TACK WELDS, AIR-ARC GOUGING AND FLAME CUTTING SHALL NOT BE PERFORMED WITHOUT ADEQUATE PREHEAT OR INCORPORATION INTO THE FINAL WELD.

THE FILLER METAL MANUFACTURER'S PUBLISHED RECOMMENDATIONS SHALL BE THE BASIS FOR DETERMINING THE ALLOWABLE RANGE OF ESSENTIAL VARIABLES FOR THE PRE QUALIFIED WPS. UNLESS NOTED OTHERWISE ON THE PLANS, BACK-UP BARS FOR CJP WELDS SHALL BE REMOVED FOLLOWED BY BACKGOUGING AND BACKWELDING.

SUBMITTALS
SUBMITTALS FOR THE ENGINEERS REVIEW WILL BE REQUIRED AS FOLLOWS:
1. CONCRETE MIX DESIGNS.
2. REINFORCING STEEL SHOP DRAWINGS.
3. STRUCTURAL STEEL AND MISCELLANEOUS METALS SHOP DRAWINGS.
4. WELDING PROCEDURE SPECIFICATIONS (AND PQR IF APPLICABLE).

CONTRACTOR SHALL SUBMIT TWO SETS OF PRINTS FOR REVIEW. FABRICATOR SHALL NOT PROCEED NOR SUBMIT TO CITY OFFICIAL UNTIL SUBMITTALS HAVE BEEN REVIEWED AND STAMPED BY THE ENGINEER.

EARTHWORK/FOUNDATION
ALL FOUNDATION DESIGN & WORK SHALL BE IN STRICT ACCORDANCE TO THE SOIL REPORT. IT IS THE CONTRACTOR'S RESPONSIBILITY TO GET THE LATEST REPORT INCLUDING THE LATEST AMENDMENTS IF ANY. THE FOLLOWING SOILS REPORT SHALL BE REFERENCED FOR THIS PROJECT:

1. COMPANY NAME: DEVO CONSULTING GEOTECHNICAL ENGINEERS.
2. REQUIREMENTS SET FORTH BY SOILS REPORT SHALL TAKE PRECEDENCE OVER THE STRUCTURAL NOTES AND DETAILS
3. DRAWINGS SHALL BE PROVIDED TO GEOTECHNICAL ENGINEER FOR REVIEW PRIOR TO BEGINNING CONSTRUCTION LETTER OF APPROVAL SHALL BE SUBMITTED TO BUILDING DEPARMENT.
4. GEOTECHNICAL ENGINEER SHALL BE RETAINED TO PROVIDE OBSERVATION & TESTING SERVICES DURING THE GRADING & FOUNDATION PHASE OF CONSTRUCTION PER THE SOILS REPORT RECOMMENDATIONS AND, INSPECTION AND TESTING REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENTS.

REQUIRED OBSERVATIONS BY STRUCTURAL ENGINEER OF RECORD:

1. FOUNDATION REINF
2. STEEL FRAMING

CONTRACTOR SHALL NOTIFY ENGINEER A MINUMUM OF 2 WORKING DAYS PRIOR TO THE TIME WHEN HIS PRESENCE IS REQUIRED. PLEASE NOTE THAT THESE OBSERVATIONS ARE INDEPENDENT OF INSPECTIONS REQUIRED BY THE CITY BUILDING DEPARTMENT.

TESTING AND SPECIAL INSPECTION

SPECIAL INSPECTIONS

CONCRETE
1. CONTINUOUSLY INSPECT THE PLACEMENT OF ALL CONCRETE.
2. SAMPLE CONCRETE: ASTM C172, EXCEPT SLUMP SHALL COMPLY WITH ASTM C94.
3. TEST SLUMP: ASTM C143, ONE TEST AT POINT OF TRUCK DISCHARGE FOR 50 CY OR FRACTION THEREOF FOR EACH TYPE OF CONCRETE± ADDITIONAL TESTS REQUIRED WHEN CONCRETE CONSISTENCY SEEMS TO HAVE CHANGED.
4. TAKE COMPRESSION TEST SPECIMENS: ASTM C31, TAKE ONE SET OF 3 STANDARD CYLINDERS FOR EACH 50 CY OF CONCRETE OR FRACTION THEREOF FOR EACH TYPE OF CONCRETE TAKEN EACH DAY. MOLD AND STORE CYLINDERS FOR LABORATORY CURED TEST SPECIMENS EXCEPT WHEN FIELD-CURE TEST SPECIMENS ARE REQUIRED.
5. TEST COMPRESSIVE STRENTH: ASTM C39± ONE SPECIMEN TESTED AT 7 DAYS, TWO SPECIMENS TESTED AT 28 DAYS.

REINFORCING STEEL
1. VERIFY THAT MILL CERTIFICATES SHOW REINFORCING STEEL IS IN COMPLIANCE WITH PROJECT SPECIFICATIONS.
2. TAKE A 5' LONG SAMPLE OF EACH BAR SIZE FROM EACH HEAT FOR EACH TYPE OF REINFORCING STEEL SHALL BE TESTED FOR ULTIMATE STRENGTH, YIELD STRESS, MODULUS OF ELASTICITY AND PERCENT ELONGATION AT RUPTURE.
3. PERIODICALLY INSPECT THE PLACEMENT OF REINFORCING STEEL FOR CONCRETE WHICH IS REQUIRED TO HAVE CONTINUOUS INSPECTION.

NON-SHRINK/EXPANSIVE GROUT
TAKE TEST SPECIMENS AND CONTINUOUSLY INSPECT THE PLACEMENT OF NON-SHRINK/EXPANSIVE GROUT.

BOLTS INSTALLED IN CONCRETE
PERIODICALLY INSPECT INSTALLATION OF BOLTS AND CONTINUOUSLY INSPECT PLACEMENT OF CONCRETE AROUND SUCH BOLTS.

METAL DECKING
1. VERIFY THAT MILL CERTIFICATES SHOW METAL DECKING TO BE IN COMPLIANCE WITH PROJECT SPECIFICATIONS.
2. PERIODICALLY INSPECT THE PLACEMENT OF ALL METAL DECKING PRIOR TO METAL DECKING BEING COVERED.

STRUCTURAL STEEL AND MISCELLANEOUS IRON
VERIFY THAT MILL CERTIFICATES SHOW STRUCTURAL STEEL AND MISCELLANEOUS IRON IS IN COMPLIANCE WITH PROJECT SPECIFICATIONS.

HIGH STRENGTH BOLTING
PRIOR TO BOLTING OPERATIONS:
1. VERIFY THAT MILL CERTIFICATES SHOW THAT BOLTS, NUTS AND WASHERS COMPLY WITH THE PROJECT SPECIFICATIONS.
2. VERIFY THAT THE METHODS OF TIGHTENING TO BE USED BY THE CONTRACTOR COMPLY WITH AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" - CURRENT EDITION APPROVED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS OF THE ENGINEERING FOUNDATION (RCSC) AND ENDORSED BY AISC.
3. PERFORM TENSION CALIBRATION TESTS WITH THE CONTRACTOR'S TOOLS ON 3 BOLT ASSEMBLIES FOR EACH DIAMETER, LENGTH, GRADE, PRODUCTION LOT AND TIGHTENING METHOD TO BE USED ON THE PROJECT INCLUDING PRETENSIONED ANCHOR RODS. TESTS SHALL BE PER RCSC SPECIFICATIONS.
4. INSPECT THE FAYING SURFACES OF EACH BOLTED CONNECTION FOR COMPLIANCE WITH RCSC SPECIFICATIONS.

DURING BOLTING OPERATIONS:
1. VERIFY THAT ALL PLIES OF CONNECTED MATERIAL HAVE BEEN DRAWN TOGETHER TO A SNUG CONDITION AS DEFINED PER RCSC SPECIFICATIONS BEFORE FINAL TIGHTENING.
2. VERIFY PLACEMENT OF MATCH-MARKS OR WRENCH CALIBRATION AS REQUIRED.
3. CONTINUOUSLY INSPECT FINAL TIGHTENING OPERATIONS PER RCSC SPECIFICATIONS.

EXCEPTIONS:
THE SPECIAL INSPECTOR NEED NOT BE PRESENT DURING ALL FINAL TIGHTENING OPERATIONS PROVIDED IT CAN BE VERIFIED THAT PROPER PROCEDURES WERE FOLLOWED (I.E. THE USE OF DTI's, "TWIST-OFF" TYPE BOLTS OR MATCH MARKING).

WELDING
VERIFY WELDER CERTIFICATIONS, COMPLIANCE WITH WELDING PROCEDURE SPECIFICATIONS AND PQR (IF APPLICABLE).
CONTINUOUSLY INSPECT ALL STRUCTURAL WELDING, INCLUDING WELDING OF REINFORCING STEEL.

EXCEPTIONS:
1. FLOOR AND ROOF DECK WELDING MAY HAVE PERIODIC INSPECTION.
2. WELDED STUDS USED FOR DIAPHRAGM OR COMPOSITE CONSTRUCTION MAY HAVE PERIODIC INSPECTION.
3. WELDED SHEET STEEL FOR COLD FORMED STEEL FRAMING MAY HAVE PERIODIC INSPECTION.
4. WELDED OF STAIRS AND RAILING SYSTEMS MAY HAVE PERIODIC INSPECTION.

NONDESTRUCTIVE TESTING:
1. ALL COMPLETE JOINT PENETRATION WELDS ON MATERIALS 5⁄16" THICK AND GREATER REQUIRE ULTRASONIC TESTING. THE RATE OF TESTING MAY BE REDUCED TO 25% IF THE FAILURE RATE MEETS THE REQUIREMENTS OF FBC AND AWS SECTION 6.20.
2. PARTIAL JOINT PENETRATION WELDS WITH EFFECTIVE THROAT THICKNESS OF ¾" OR GREATER REQUIRE ULTRASONIC TESTING.
3. WELDS ON BASE METAL THICKER THAN ½" REQUIRE ULTRASONIC TESTING.

STRUCTURAL MASONRY
DURING THE PREPARATION OF MASONRY WALL PRISMS, INSPECTION OF GROUT SPACE IMMEDIATELY PRIOR TO CLOSING OF CLEANOUTS AND PERIODICALLY DURING THE PLACEMENT OF REINFORCING AND GROUTING OPERATIONS.

SPECIAL INSPECTOR
THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE HIS COMPETENCE TO THE SATISFATCTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF A CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR
THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPLICABLE DESIGN DRAWINGS AND SPECIFICATIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE ENGINEER OR ARCHITECT OF RECORD, AND ANY OTHER DESIGNATED PERSONS ON A WEEKLY BASIS. ALL DISCREPANCIES SHALL SE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE PROPER DESIGN AUTHORITY AND TO THE BUILDING OFFICIAL. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WEATHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF HIS KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISION OF THIS CODE.

GENERAL NOTES

1. CONSIDER GENERAL NOTES AS APPLYING ALL DRAWINGS.
2. DO ALL WORK IN ACCORDANCE WITH ALL STATE AND LOCAL BUILDING CODES IN EFFECT AT PLACE AND TIME OF CONSTRUCTION.
3. PROVIDE SPECIAL INSPECTION AS REQUIRED BY STRUCTURAL SPECIFICATIONS.
4. CONSTRUCT THOSE FEATURES OF THE PROJECT, WHICH MAY NOT BE FULLY SHOWN, IN MANNER SIMILAR TO THAT USED FOR SIMILAR FEATURES.
5. OMISSION OR CONFLICTS BETWEEN VARIOUS ELEMENTS OF THE DRAWING, NOTES AND DETAILS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND RESOLVED PRIOR TO THE PROCEEDING WITH THE WORK.
6. CONTRACTOR SHALL REVIEW THE NEED FOR TEMPORARY , SHORING, CHEMICAL GROUTING OR UNDERPINNING PRIOR TO EXCAVATION. CONTRACTOR SHALL DESIGN AND INSTALL ALL TEMPORARY BRACING, ETC.. REQUIRED DURING ALL STAGES OF WORK.
7. CONTRACTOR SHALL SUBMIT IN WRITING, ANY REQUEST FOR MODIFICATIONS TO THE PLANS AND SPECIFICATIONS. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING REQUESTED.
8. ALL CONSTRUCTION WORK SHALL CONFORM TO 2010 FLORIDA BUILDING CODE.
9. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS IN THE FIELD PRIOR TO ORDERING MATERIALS OR STARTING CONSTRUCTION AND NOTIFY ENGINEER IMMEDIATELY OF ANY DISCREPANCIES.
10. RETURN TO ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF OPENINGS AND PENETRATIONS. COORDINATE PENETRATIONS. NO NEW OPENING SHALL BE MADE WITHOUT WRITTEN APPROVAL FROM THE ARCHITECT.
11. REFER TO ARCH, MECH, AND ELECTRICAL DRAWINGS FOR LOCATION AND SIZE OF BLOCK OUT, INSERTS, OPENINGS, AND CURBS. DIMENSIONS ARE NOT SHOWN ON STRUCTURAL DRAWINGS.
12. GENERAL CONTRACTORS SHALL VERIFY WITH STRUCTURAL ENGINEER ALL MECH. UNIT LOCATIONS PRIOR TO INSTALLATIONS.
13. THE SHOP DRAWINGS AND FABRICATION OF THE CURTAIN WALL/STOREFRONT SYSTEM TO BE BY THE CURTAIN WALL MANUFACTURER. THE INSTALLER IS NOT CONSIDEREED ACCEPTABLE AS THE MANUFACTURER.
14. CONSTRUCTION LIABILITY, CONSTRUCTION CONTRACTOR AND HIS SUBCONTRACTORS AGREE THAT IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONTRACTOR AND HIS SUBCONTRACTORS WILL BE REQUIRED TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONTRACTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THAT THIS REQUIREMENT SHALL BE MADE TO APPLY CONTINUOUSLY AND NOT LIMITED TO NORMAL WORKING HOURS, AND CONSTRUCTION CONTRACTOR AND HIS SUBCONTRACTORS FURTHER AGREE TO DEFEND, INDEMNIFY AND HOLD DESIGN PROFESSIONAL HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPT LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF DESIGN PROFESSIONAL.

ABBREVIATIONS

STANDARD SIERRA ENGINEERING GROUP STRUCTURAL ABBREVIATIONS

ABBREVIATION	ABBR	HOLDOWN	HD
ABOVE FINISH FLOOR	AFF	HORIZONTAL	HORIZ
ADDITIONAL	ADD'L	HOT DIPPED	HDG
ALTERNATE	ALT	GALVANIZED	
ANCHOR BOLTS	AB	INSIDE DIAMETER	ID
ARCHITECTURAL	ARCH	MACHINE BOLT	MB
ATTACHMENT	ATTACH	MANUFACTURER	MFR
BEAM	BM	MATERIAL	MAT'L
BEARING	BRG	MAXIMUM	MAX
BETWEEN	B'TWN	MECHANICAL	MECH
BLOCKING	BLK'G	METAL	MTL
BOTTOM	BOTT	MICROLLAM	ML
BOTTOM OF	BO	MINIMUM	MIN
CAMBER	CAMB	NEAR SIDE	NS
CEILING	CLG	NEARSIDE AND FAR	
CHANGE	CHG	SIDE	NSFS
CHANNEL	C	ON CENTER	OC
CLEARANCE	CLR	OPEN WEB JOIST	O W J
COLLECTOR	COLL	OPENING	OPNG
COLUMN	COL	OUTSIDE DIAMETER	OD
CONCRETE	CONC	OVER	O/
CONCRETE MASONRY UNIT	CMU	PARALLAM	PARA
CONDENSING UNIT	CU	PLATE	⅊
CONNECTION	CONN	PLYWOOD	PLYWD
CONTINUOUS	CONT	POLYETHYLENE	PE
DEEP	DP	PRESSURE TREATED	
DIAGONAL	DIAG	DOUGLAS FIR	PTDF
DIAMETER	DIA	RAFTER	RFTR
DIMESION	DIM	REINFORCEMENT	REINF
DOUBLE	DBL	REQUIRED	REQ'D
DRAWING	DWG	ROOF TOP UNIT	RTU
DRAWINGS	DWGS	SEE ARCH DRAWING	SAD
EACH	EA	SELF DRILLING SCREWS	SDS
EACH WAY	EW	SHEATHING	SHTG
EDGE NAILING	EN	SHEET	SHT
ELECTRICAL	ELEC	SIMILAR	SIM
ELEVATION	ELEV	SLAB ON GRADE	SOG
EMBEDMENT	EMBED	STANDARD	STD
EQUAL	EQ	STEEL	STL
EVERY OTHER	E/O	STIFFENER	STIF
EXISTING	(E)	STRINGER	STRNG'R
FAR SIDE	FS	STRUCTURAL	STRUCT
FINISH FLOOR	FF	TAPERED STEEL	
FLOOR	FLR	GIRDER	TSG
FLOORJOIST	FJ	THREADED ROD	THRDRD
FOOTING	FTG	TOUNGE AND GROOVE	T&G
FOUNDATION	FND	TOP AND BOTTOM	T&B
FRAMING	FRM'G	TOP OF	TO
GALVANIZED	GALV	TRUSS JOIST	TJI
GAUGE	GA	TUBE STEEL	TS
GIRDER	GDR	TYPICAL	TYP
GLUE-LAM	GLB	UNLESS NOTED	
GYPSUM WALL BOARD	GWB	OTHERWISE	UNO
HANGER	HGR	VERIFY-IN-FIELD	VIF
HEADED WELDED STUD	HWS	VERTICAL	VERT
HEADER	HDR	WELDED-WIRE-FABRIC	WWF
HEIGHT	HT	WIDE FLANGE	W
HIGH STRENGTH	HS	WITH	W/
HIGH STRENGTH BOLT	HSB	WITHOUT	W/O
HOLLOW STEEL SECTION	HSS		

PROJECT DATA

1. PLANS AND CALCULATIONS FOR THE STRUCTURAL DESIGN WERE BASED UPON: THE 2020 FLORIDA BUILDING CODE.
2. VERTICAL:
   GROUND FLOOR LIVE LOAD = 100 PSF
   ROOF LIVE LOAD = 30 PSF
   RAIN LOAD 4IN FLOOD = 21 PSF
   ROOF DEAD LOAD = 27 PSF
3. WIND:
   DESIGN WIND SPEED = 135 MPH
   EXPOSURE = C
   A. COMPONENTS & CLADDING
      DESIGN PRESSURE = SEE SHEETS S1.1

SHEET INDEX

hmd GROUP PA architects
10556 n.w. 26 street, suite d-101 doral, fl 33172
phone 305.594.2975 fax 305.594.2979 email hmdgroup@bellsouth.net
www: hmdgrouparchitects.com
ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING
WILFREDO F. PEREZ
FLORIDA LIC. NO. 13418
LICENSE # AA F000074

ARCHITECTS PROJECT #: 22-0052

SIERRA ENGINEERING GROUP
TEL:305.665.5445
5825 SUNSET DRIVE, SUITE 200
SOUTH MIAMI, FLORIDA 33143
FAX:305.665.5469
C.A.#. 9830
JESUS F. SIERRA
FLORIDA P.E.# 51132

Yard House

Issue Date: 07-29-2022

REVISION INFORMATION

Restaurant #: 8352

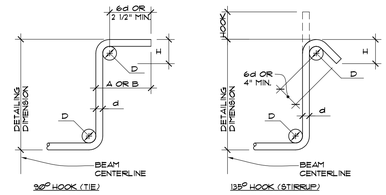
8367 International Drive

Orlando, FL

STRUCTURAL SPECIFICATIONS & GENERAL NOTES

S0.0

22046.0



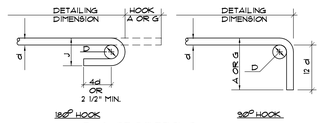
90° HOOK (TIE) 135° HOOK (STIRRUP)

STIRRUP OR TIE SIZE	D	90° HOOK	135° HOOK	
		HOOK A OR G	HOOK A OR G	H APPROX.
#3	1 1/2"	4"	4"	2 1/2"
#3	2"	4 1/2"	4 1/2"	3"
#3	2 1/2"	6"	5 1/2"	3 3/4"

STIRRUP OR TIE STANDARD HOOKS

NTS

A S0.1



180° HOOK 90° HOOK

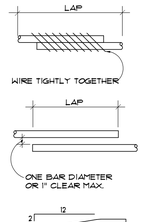
D = 6d FIR #3 THROUGH #8
D = 8d FOR #9, #10, AND #11
D = 10d FOR #14 AND #18

BAR SIZE	180° HOOK		90° HOOK
	A OR G	J	A OR G
#3	5"	3"	6"
#4	6"	4"	8"
#5	7"	5"	10"
#6	8"	6"	1'-0"
#7	10"	7"	1'-2"
#8	11"	8"	1'-4"
#9	1'-3"	11 3/4"	1'-7"
#10	1'-5"	1'-1 1/4"	1'-10"
#11	1'-7"	1'-2 3/4"	2'-0"
#14	2'-3"	1'-9 3/4"	2'-7"
#18	3'-0"	2'-4 1/2"	3'-5"

REBAR STANDARD HOOKS

NTS

B S0.1



BAR SIZE	CONCRETE		MASONRY
	MINIMUM LAP LENGTH FOR REINFORCING BARS (UNLESS OTHERWISE NOTED)		
	LAP : FT.-IN.		LAP : FT.-IN.
	TOP BARS	OTHER BARS	ALL BARS
#3	1'-9"	1'-3"	1'-1"
#4	2'-5"	1'-8"	2'-1"
#5	3'-0"	2'-2"	2'-8"
#6	3'-10"	2'-8"	3'-2"
#7	5'-3"	3'-8"	3'-8"
#8	6'-10"	5'-0"	4'-2"
#9	8'-8"	6'-3"	4'-8"
#10	11'-0"	7'-11"	5'-4"
#11	13'-7"	9'-8"	5'-11"

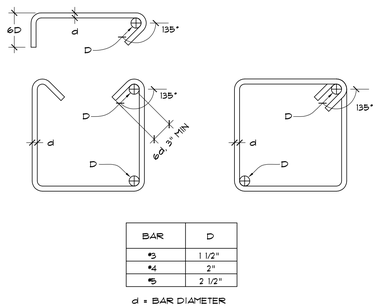
NOTES:

1. TOP BARS ARE HORIZONTAL BARS SO PLACED THAT MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE BARS.
2. LAP LENGTHS ARE BASED ON F'c = 3000 PSI AND Fy = 60000 PSI AND Fm=1500 PSI

TYPICAL REBAR SPLICES

NTS

C S0.1



BAR	D
#3	1 1/2"
#4	2"
#5	2 1/2"

d = BAR DIAMETER

STIRRUP AND TIE DETAILS

NTS

D S0.1

hmd GROUP PA architects
10556 nw 26 street, suite p-101 doral, fl 33172
phone 305.594.2315 fax 305.594.2379 email hmdgroup@bellsouth.net
www.hmdgroupparchitects.com
ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING
HILBERTO F. PEREZ
FLORIDA LIC. NO. 13418
LICENSE # AA-P000774

ARCHITECTS PROJECT #: 22-0052

SIERRA ENGINEERING
G R O U P
TEL:305.665.5445
5825 SUNSET DRIVE, SUITE 200
SOUTH MIAMI, FLORIDA 33143
FAX:305.665.5469
C.A.#. 9830
JESUS F. SIERRA
FLORIDA P.E.# 51132

Yard House

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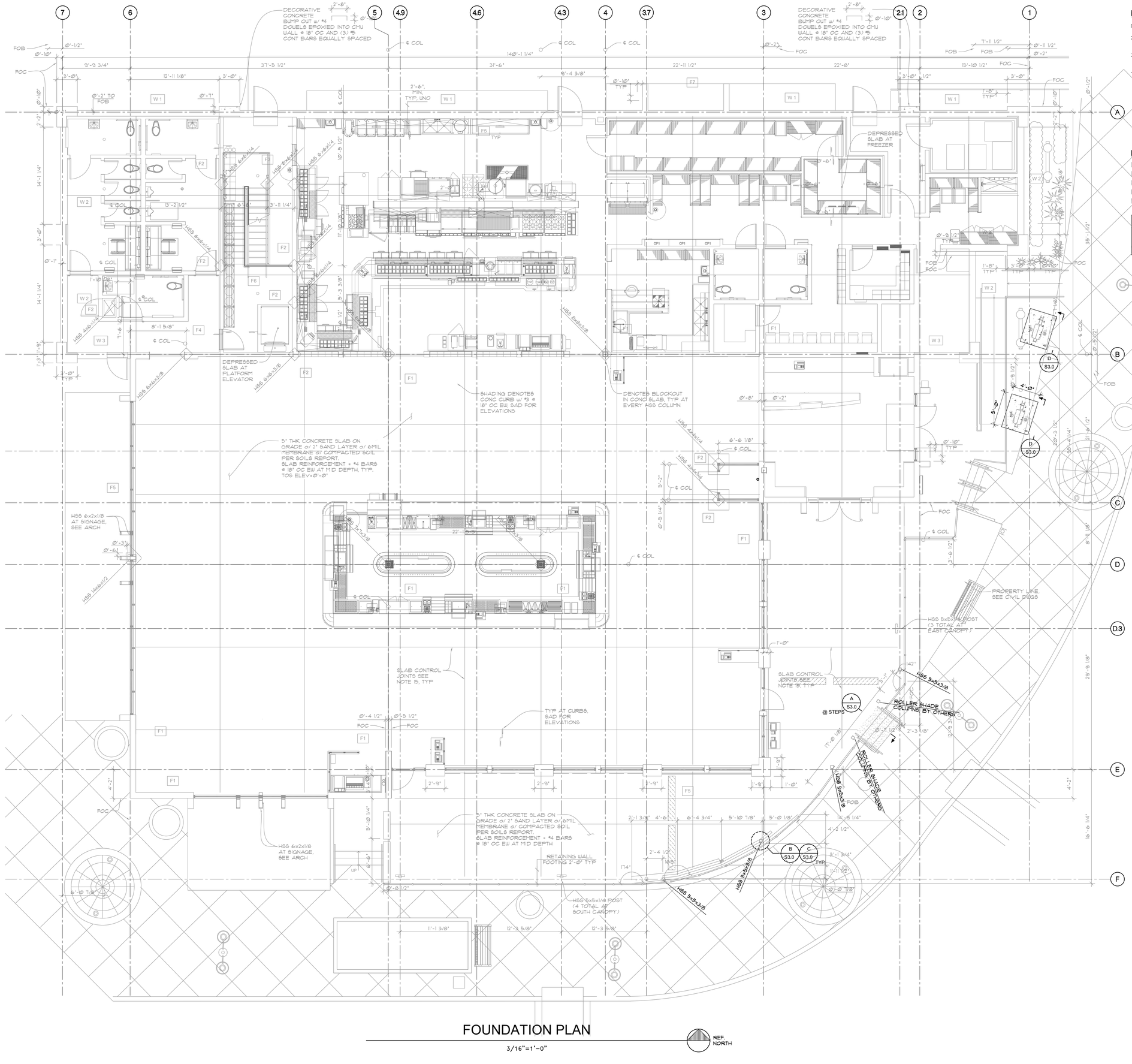
8367 International Drive

Orlando, FL

CONCRETE TYPICAL DETAILS

S0.1

22046.0



FOUNDATION PLAN

3/16"=1'-0"

REF. NORTH

NOTES:

1. DO NOT SCALE DRAWINGS. SCALE FOR REFERENCE ONLY
2. SEE SHEETS S000, S001, & S002 FOR SPECIFICATIONS AND TYPICAL DETAILS.
3. VERIFY ALL DOOR AND WINDOW OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO BEGINNING CONSTRUCTION.
4. VERIFY ALL PENETRATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO BEGINNING CONSTRUCTION.

LEGEND

- (E) FULLY GROUTED CMU WALLS, TYP UNO
- (E) MTL STUD WALLS
- (E) CONC WALLS & PIERS, TYP UNO
- (E) CONC CURB, SAD FOR ELEVATIONS
- (E) SILL CMU WALL

BASE PLATE SCHEDULE

MARK	COLUMN SIZE	BASE PLATE SIZE	ANCHOR BOLTS (+)DIM/CONC EMBEDMENT	BOLT HOLE GRID
BP1	HSS 5"x5"x3/8"	11"x11"x5/8"	(4) 5/8"x12"	8"x8"

hmd GROUP PA architects
10556 nw 26 street, suite p-101 doral, fl 33172
phone 305.594.2315 fax 305.594.2979 email hmd@hmdgrouparchitects.com
ARCHITECTURE INTERIOR DESIGN CORPORATE PLANNING
HILARIO D. PEREZ FLORIDA LIC. NO. 13418 LICENSE # AA 0000774

ARCHITECTS PROJECT #: 22-0094

SIERRA ENGINEERING GROUP
TEL:305.665.5445
5825 SUNSET DRIVE, SUITE 200
SOUTH MIAMI, FLORIDA 33143
FAX:305.665.5469
C.A.#. 9830
JESUS F. SIERRA
FLORIDA P.E.# 51132

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REVISION INFORMATION
1
2
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Restaurant #: 8352

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Orlando, FL

CANOPY FOUNDATION FLOOR PLAN

S1.0

22046.0

JESUS F. SIERRA LICENSE No. 51132 STATE OF FLORIDA PROFESSIONAL ENGINEER

9-22-23

NOTES:

1. DO NOT SCALE DRAWINGS. SCALE FOR REFERENCE ONLY
2. SEE SHEETS S0.0, & S0.1 FOR SPECIFICATIONS AND TYPICAL DETAILS.
3. VERIFY ALL DOOR AND WINDOW OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO BEGINNING CONSTRUCTION.
4. VERIFY ALL PENETRATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO BEGINNING CONSTRUCTION.
5. FOR ROOF METAL DECK WELDING SCHEDULE, REFER TO (K / S4.0)
6. BOTTOM OF DECK ELEVATION 15'-8", TYP.
7. --- DENOTES BOTTOM OF DECK ELEVATION.

LEGEND

(E) CMU WALLS, TYP UNO
(E) MTL STUD WALLS
(E) CONC WALLS & PIERS, TYP UNO

WIND PRESSURE TABLE		
ZONE	POSITIVE (PSF)	NEGATIVE (UPLIFT) (PSF)
1	31	-31

DECORATIVE CONCRETE BUMP OUT w/ #4 DOWELS EPOXIED INTO CMU WALL @ 18" OC AND (3) #5 CONT BARS EQUALLY SPACED

DEPRESSED SLAB AT PLATFORM ELEVATOR

DEPRESSED SLAB AT FREEZER

SHADING DENOTES CONC CURB w/ #3 @ 18" OC EW, SAD FOR ELEVATIONS

DENOTES BLOCKOUT IN CONC SLAB, TYP AT EVERY HSS COLUMN

5" THK CONCRETE SLAB ON GRADE o/ 2" SAND LAYER o/ 6MIL MEMBRANE o/ COMPACTED SOIL PER SOILS REPORT. SLAB REINFORCEMENT = #4 BARS @ 18" OC E.W. AT MID DEPTH, TYP. TOS ELEV=Ø'-Ø"

SLAB CONTROL JOINTS SEE NOTE 15, TYP

TYP AT CURBS, SAD FOR ELEVATIONS

DENOTES 1 1/2"-20GA (PLN36) METAL ROOF DECK FOR DECK ATTACHMENT SCHEDULE, SEE (K / S4.0)

CONT HSS 14x4x1/4 OUTSIDE FACE OF COLUMN

CONT HSS 14x4x1/4 HSS 14x4x3/16 BELOW @ OUTSIDE FACE OF COLUMN

SOLDER SPACE COVERS BY OTHERS

RETAINING WALL FOOTING 2'-0" TYP.

HSS 6x2x1/8 AT SIGNAGE, SEE ARCH

HSS 5x5x1/4 POST (4 TOTAL AT SOUTH CANOPY)

HSS 5x5x1/4 POST (3 TOTAL AT EAST CANOPY)

PROPERTY LINE, SEE CIVIL PLANS

MEZZANINE FRAMING PLAN

3/16"=1'-0"

REF. NORTH

WIND PRESURES

1/16"=1'-0"

hmd GROUP PA architects
10556 nw 26 street, suite p-101 doral, fl 33172
phone 305.424.2315 fax 305.594.2979 email hmd@hmdgrouparchitects.com
ARCHITECTURE INTERIOR DESIGN CORPORATE PLANNING
HILARIO D. PEREZ FLORIDA LIC. NO. 13418 LICENSE # AA 0003074

ARCHITECTS PROJECT #: 22-0052

SIERRA ENGINEERING GROUP
TEL:305.665.5445
5825 SUNSET DRIVE, SUITE 200
SOUTH MIAMI, FLORIDA 33143
FAX:305.665.5469
C.A.#. 9830
JESUS F. SIERRA
FLORIDA P.E.# 51132

Yard House

Issue Date: 07-29-2022

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Restaurant #: 8352

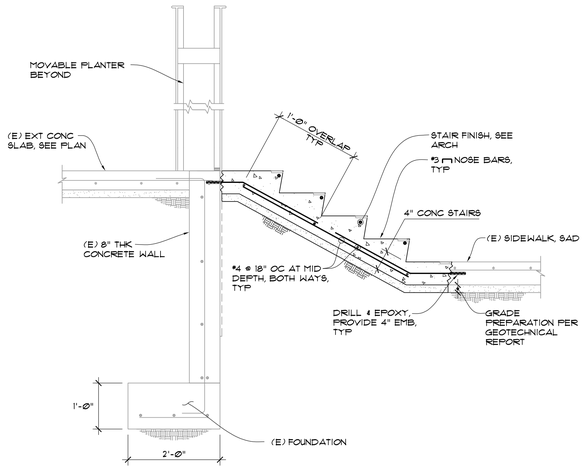
8367 International Drive

Orlando, FL

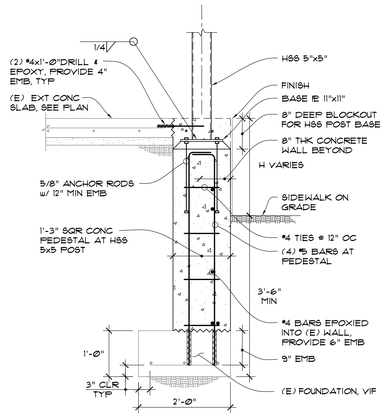
CANOPY ROOF PLAN

S2.0

22046.0



SECTION A
3/4"=1'-0"
S3.0



SECTION B
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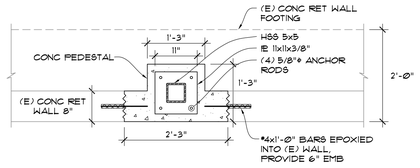
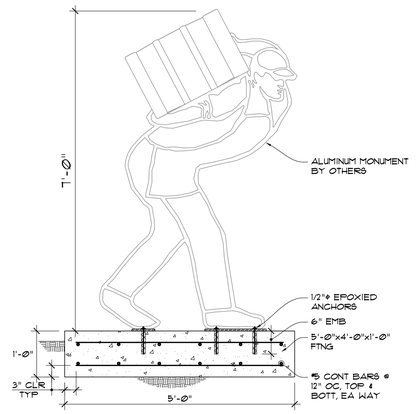


SECTION C
3/4"=1'-0"
S3.0



SECTION D
3/4"=1'-0"
S3.0

hmd GROUP PA
architects
10656 nw 26 street, suite st-101 doral, fl 33172
phone 305.594.2315 fax 305.594.2375 email hmdgroup@bellsouth.net
www.hmdgroupparchitects.com
ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING
HILBERTO F. PEREZ
FLORIDA LIC. NO. 13418
LICENSE # AA P000074

ARCHITECTS PROJECT #:
22-0052

SIERRA
ENGINEERING
G R O U P
TEL:305.665.5445
5825 SUNSET DRIVE, SUITE 200
SOUTH MIAMI, FLORIDA 33143
FAX:305.665.5469
C.A.#. 9830
JESUS F. SIERRA
FLORIDA P.E.# 51132

Yard House

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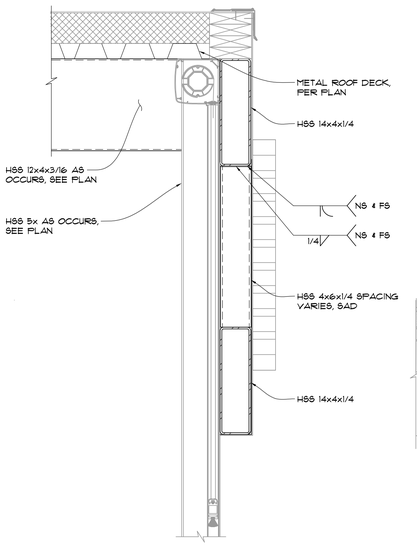
8367 International Drive

Orlando, FL

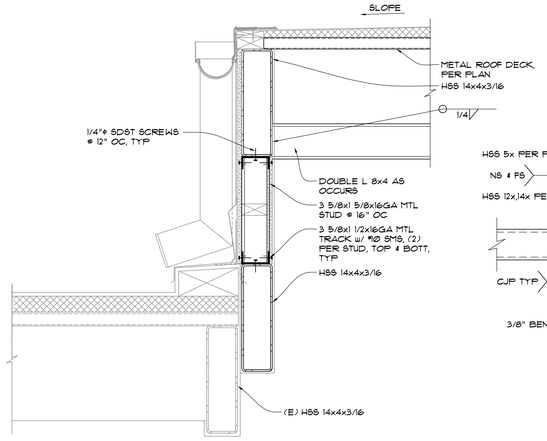
DETAILS

S3.0

22046.0



SECTION 1 1/2"=1'-0" A S4.0



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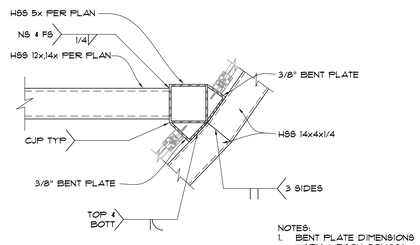

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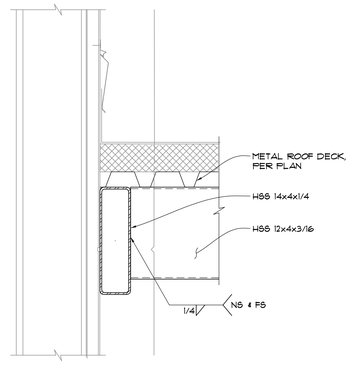

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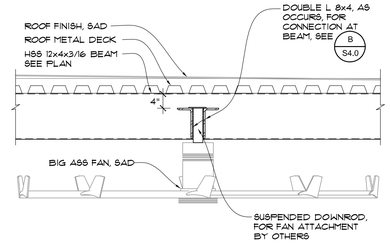

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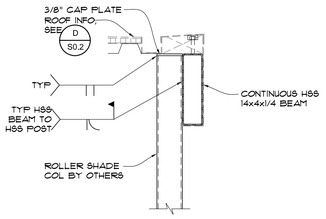

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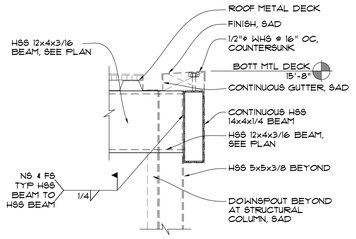

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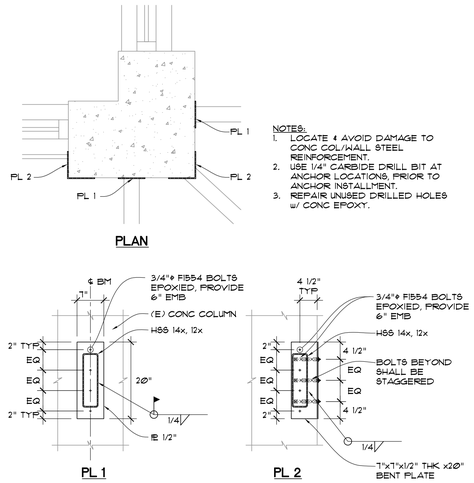

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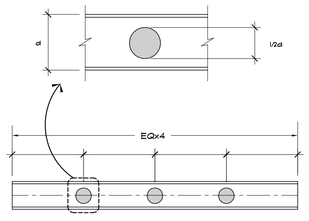


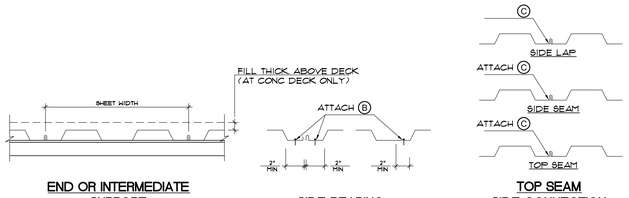
TYPICAL BEAM SPAN NTS

NOTES

1. BEAM PENETRATIONS NOT TO EXCEED 1/2 BEAM DEPTH
2. BEAM PENETRATIONS TO OCCUR WHERE NOTED
3. PENETRATIONS TO BE TORCH CUT ¢ GROUND SMOOTH
4. ANY REQUEST FOR MODIFICATION OF PENETRATION PROCEDURE OR SIZES TO BE MADE IN WRITING
5. PENETRATIONS NOT TO OCCUR IN STEEL MOMENT FRAMES

TYPICAL MECHANICAL PENETRATIONS AT STEEL BEAMS
NTS

J S4.0



TYPICAL STEEL DECK ATTACHMENT DETAILS

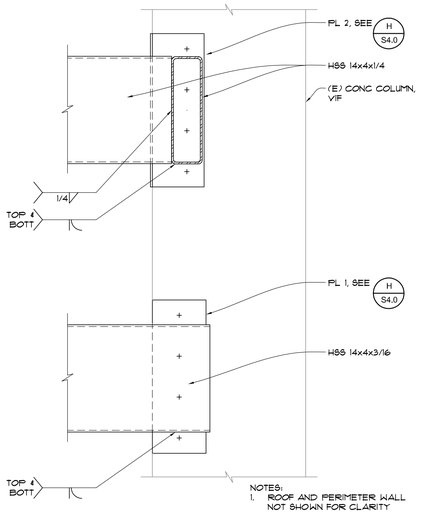
DECK PROFILE	DECK ATTACHMENT						LOCATION	FILL THICKNESS ABOVE DECK
	END ¢ INTERMEDIATE BEARING (A)		SIDE BEARING (B)		SIDE CONNECTION (C)			
	SIZE ¢ TYPE	# PER SHEET	SIZE ¢ TYPE	SPACING	SIZE ¢ TYPE	SPACING		
VERCO PLN-36	5/8"ø PUDDLEWELD	4	5/8"ø PUDDLEWELD	12" OC	5/8"x 1/2" SEAM WELD	12" OC	ROOF DECK	6" INSULATION

NOTES:

1. ATTACHMENT DETAILS ARE ALSO APPLICABLE AT WIDE FLANGES ANGLES AND HSS WHERE THEY OCCUR. ATTACHMENT OF DECK TO HAT CHANNELS AND HAT CHANNELS TO SUPPORT SHALL CONFORM TO SCHEDULE ABOVE.
2. IF CONC INSULATION WILL BE USED OVER ROOF DECK, IT SHOULD BE AN AGGREGATE TYPE THAT CONFORM TO SPECIFICATION LISTED IN ICBO ES REPORT - 2018P OR INSULATION SYSTEM APPROVED BY ARCHITECT.
3. 1/2" PUDDLE WELDS MAY BE USED IN LIEU OF 3/8"x1" PUDDLE WELDS TYP.
4. WELDING OF DECK TO HAT CHANNELS AND HAT CHANNELS TO SUPPORT SHALL CONFORM TO SCHEDULE ABOVE, AND N.O.A. N° 03.0923.04
5. MAX DESIGN PRESSURE = 30 PSF
6. MECHANICAL ATTACHMENTS INSTEAD OF WELDING MAY BE USED IN PLB-36 DECK ONLY.

DECK ATTACHMENT SCHEDULE
NTS

K S4.0



SECTION 1 1/2"=1'-0" L S4.0

hmd GROUP PA architects
10556 nw 26 street, suite p-101 doral, fl 33172
phone 305.594.2315 fax 305.594.2375 email hmdgroup@bellsouth.net
www.hmdgroupparchitects.com
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SOUTH MIAMI, FLORIDA 33143
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Restaurant #: 8352

8367 International Drive

Orlando, FL

MAIN ROOF DETAILS

S4.0

22046.0

DEVITA
ATLANTA • CHARLOTTE • GREENVILLE • RICHMOND
877.4.DEVITA • corp@devitainc.com
DeVita & Associates, Inc. Project : 22026-02
Drawn By: WJS Checked By: KJM
FL Firm License # 9687

STORM DRAIN CALCULATION

THE PROJECT IS INSTALLATION OF A SEMICIRCULAR ROOF OVER A PATIO AT ONE CORNER OF THE BUILDING. THE NEW ROOF BRIDGES OVER TWO EXISTING COVERED PATIOS AND WILL DRAIN ON TO THE EXISTING ROOFS.

THE BUILDING ROOF DOES NOT DRAIN ONTO THE NEW ROOF, BUT WALL ABOVE THE NEW ROOF IS COUNTED IN THE STORM DRAIN CALCULATIONS.

RAINFALL VOLUME:

PER FLORIDA PLUMBING CODE (2020 ED.) FIGURE 1106.1:
100-YEAR LOCAL RAINFALL = 4.5 IN./HR
= 0.375 FT/HR

LEFT SIDE

THE NEW ROOF HAS A HIGH POINT AT THE CORNER OF THE BUILING, AND EACH SIDE OF THE HIGH POINT WILL DRAIN ONTO DIFFERENT EXISTING PATIO ROOF. THE LEFT SIDE OF THE NEW ROOF IS 236 SQ FT AND IT DRAINS ONTO A EXISTING 643 SQ FT ROOF. THERE IS 123 SQ FT OF WALL ABOVE THE NEW ROOF AND 419 SQ FT OF WALL DRAINING ONTO THE EXISTING ROOF. HALF OF THE WALL AREA COUNTS TOWARDS ROOF DRAINAGE AREA.

TOTAL ROOF AREA OF DRAINAGE: 236 + 643 = 879 SQ FT
WALL ABOVE ROOF: 123 + 419 = 542 SQ FT

TOTAL AREA: 879 SQ FT + (½ X 542 SQ FT) = 1150 SQ. FT

ROOF DRAIN VOLUME: 1150 SQ FT X 0.375 FT / HR = 431 CUBIC FEET / HOUR
= 7.19 CUBIC FEET / MINUTE
= 53.5 GALLON / MIN (GPM)

LEFT SIDE DOWNSPOUTS:

THE EXISTING ROOF HAS TWO 4X4 DOWNSPOUTS
PER FLORIDA PLUMBING CODE (2020 ED.): TABLES 1106.3
A 4" X 4" VERTICAL LEADER HAS A CAPACITY OF 192 GAL/MIN
THE DOWNSPOUTS WILL HAVE ADEQUATE CAPACITY TO ADD THE NEW PATIO ROOF.

RIGHT SIDE

THE RIGHT SIDE OF THE NEW ROOF IS 261 SQ FT AND IT DRAINS ONTO AN EXISTING 459 SQ FT ROOF. THERE IS 123 SQ FT OF WALL ABOVE THE NEW ROOF AND 254 SQ FT OF WALL DRAINING ONTO THE EXISTING ROOF. HALF OF THE WALL AREA COUNTS TOWARDS ROOF DRAINAGE AREA.

TOTAL ROOF AREA OF DRAINAGE: 261 + 459 = 720 SQ FT
WALL ABOVE ROOF: 123 + 254 = 377 SQ FT

TOTAL AREA: 720 SQ FT + (½ X 377 SQ FT) = 909 SQ. FT

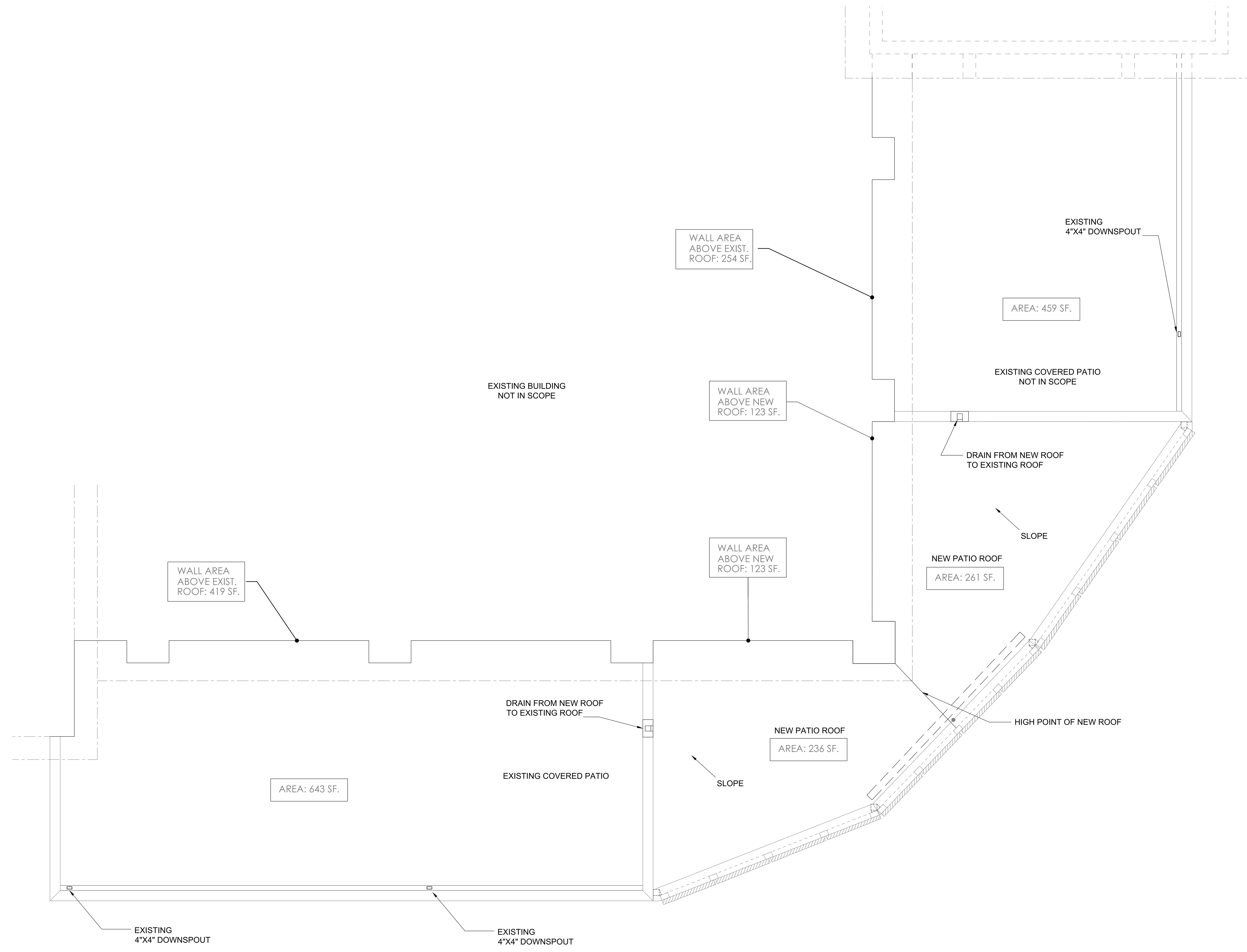
ROOF DRAIN VOLUME: 909 SQ FT X 0.375 FT / HR = 341 CUBIC FEET / HOUR
= 5.68 CUBIC FEET / MINUTE
= 42.2 GALLON / MIN (GPM)

RIGHT SIDE DOWNSPOUT:

THE EXISTING ROOF HAS ONE 4X4 DOWNSPOUT
PER FLORIDA PLUMBING CODE (2020 ED.): TABLES 1106.3
A 4" X 4" VERTICAL LEADER HAS A CAPACITY OF 192 GAL/MIN
THE DOWNSPOUT WILL HAVE ADEQUATE CAPACITY TO ADD THE NEW PATIO ROOF.

EMERGENCY OVERFLOW

THE EXISTING PATIO ROOFS HAVE 4" HIGH PARAPETS AND IN AN EMERGENCY THE WATER CAN OVERFLOW THE PARAPET. NO EMERGENCY SCUPPERS ARE REQUIRED.



1 P1.1 PLUMBING ROOF PLAN
SCALE: 1/4" = 1'-0"
0 4' 8' 12'

hmd GROUP PA architects
10556 n.w. 26 street, suite d-101 doral, fl 33172
phone 305.594.2975 fax 305.594.2979 email hmdgroup@bellsouth.net
www: hmdgrouparchitects.com
ARCHITECTURE
INTERIOR DESIGN
CORPORATE PLANNING
WILFREDO F. PEREZ
FLORIDA LIC. NO. 13418
LICENSE # AA F000074

ARCHITECTS PROJECT #: 22-0052

KEITH F. MATTISON LICENSE No. 76329 STATE OF FLORIDA PROFESSIONAL ENGINEER

Yard House

Issue Date: 08-05-2022

REVISION INFORMATION
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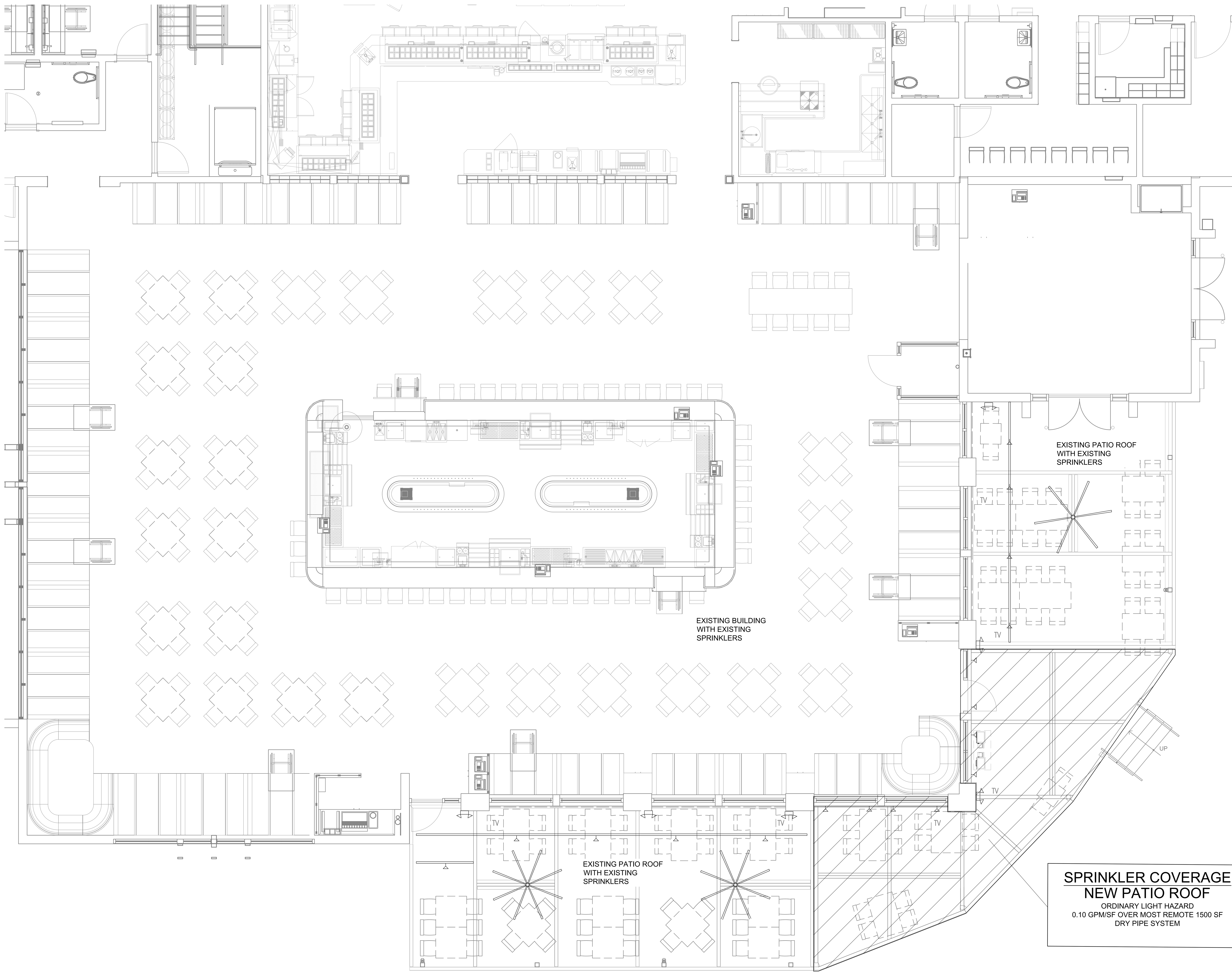
Restaurant #: 8352

8367 International Drive

Orlando, FL

PLUMBING ROOF DRAINAGE PLAN

P1.1



1 / FP1.1 PATIO FIRE PROTECTION CRITERIA
NOT TO SCALE

NOTE:

1. THIS IS AN EXISTING BUILDING WITH SPRINKLERS AND BUILDING HAS TWO EXISTING COVERED PATIOS. THIS PROJECT IS A NEW ROOF OVER AN UNCOVERED PART OF THE PATIO.
2. THE FIRE PROTECTION SCOPE IS TO EXTEND THE EXISTING SPRINKLER SYSTEM TO COVER THE NEW PATIO ROOF.
3. SPECIFICATION CALLS FOR DRY PIPE SYSTEM, BUT ALTERNATIVE APPROACH SUCH AS DRY PENDENT SPRINKLERS IS ALLOWED.

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KEITH F. MATTISON LICENSE No. 76329 STATE OF FLORIDA PROFESSIONAL ENGINEER
10/26/22

Yard House

Issue Date:	08-05-2022
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8367 International Drive

Orlando, FL

FIRE PROTECTION CRITERIA PLAN

FP1.1

GENERAL NOTES:

A. REFER TO ARCHITECTURAL REFLECTED CEILING PLAN AND ELEVATIONS FOR EXACT LIGHTING LAYOUT AND LIGHTING FIXTURE SCHEDULE.

B. ALL WIRING SHALL BE NEW AND FIXTURES SHALL BE CIRCUITED AS INDICATED. EXISTING DEVICE BOXES AND CONDUIT SCHEDULED TO BE REMOVED MAY BE REUSED IF THEY COMPLY WITH ALL REQUIREMENTS OF THE NEC, IF NEW DEVICE LOCATION COINCIDES WITH EXISTING LOCATION, AND IF PRACTICAL.

C. CONDUIT AND CIRCUIT ROUTINGS SHOWN ARE SCHEMATIC AND DIAGRAMMATIC ONLY. FIELD DETERMINE EXACT CIRCUIT ROUTING AND PROVIDE ALL NECESSARY CONDUCTORS, CONDUIT, FITTINGS, JUNCTION BOXES, AND OTHER ITEMS.

D. ALL DIMMER CIRCUITS SHALL HAVE A SEPARATE NEUTRAL. SHARING OF NEUTRALS IS NOT ALLOWED.

E. UNLESS NOTED OTHERWISE, ALL CIRCUITS SHALL BE 2#12, 1#12G, 1/2"C. MAKE ADJUSTMENTS AS REQUIRED FOR VOLTAGE DROP PER NEC.

F. FIELD DETERMINE EXACT LOCATIONS OF ALL PANELS AND CONTROLS.

G. ALL ELECTRICAL CONSTRUCTION SHALL CONFORM TO THE NATIONAL ELECTRICAL CODE, APPLICABLE NEMA, ANSI AND IEEE PUBLICATIONS, U.L. STANDARDS AND OSHA REQUIREMENTS. WORK SHALL COMPLY WITH LOCAL, COUNTY, STATE AND NATIONAL CODES HAVING JURISDICTION.

H. ELECTRICAL DEVICE INSTALLATION SHALL COMPLY WITH ACCESSIBILITY CODES ADOPTED FOR LOCAL AHJ. SPECIFICALLY; MOUNT APPLICABLE SWITCHES, RECEPTACLES, AND ENVIRONMENTAL CONTROLS SO THAT THEY ARE MOUNTED WITH THE TOP OF THE DEVICE NO HIGHER THAN FORTY-EIGHT INCHES (48") ABOVE THE FINISHED FLOOR AND THE BOTTOM OF THE DEVICE NO LOWER THAN FIFTEEN INCHES (15") ABOVE THE FINISHED FLOOR. ELECTRICAL DEVICES ABOVE A COUNTERTOP OR OTHER OBSTRUCTION SHOULD COMPLY WITH ALL APPLICABLE SECTIONS OF ICC, ANSI, AND ADA. VERSION OF CODE SHALL BE AS ADOPTED BY AHJ.

I. ALL LIGHT FIXTURES ARE EXISTING UNLESS NOTED OTHERWISE. LOCATIONS ARE INDICATED ON THIS PLAN FOR REFERENCE ONLY. FIELD VERIFY ALL LIGHTING FIXTURE LOCATIONS.

J. ABANDONED POWER WIRING WILL BE REMOVED BACK TO THE SOURCE. THE ACCESSIBLE PORTIONS OF ABANDONED CONDUIT/TUBING AND EQUIPMENT SHALL BE REMOVED. THE ACCESSIBLE PORTIONS OF ABANDONED CABLES (VOICE, DATA, VIDEO, ALARM, ETC.) SHALL BE REMOVED.

K. CONTRACTOR SHALL INSPECT SITE PRIOR TO SUBMITTING BID.

L. WIRING SHALL BE 600V, THHN/THWN, COPPER BUILDING WIRE. WIRING SHALL BE INSTALLED IN CONDUIT. CONDUIT SHALL BE EMT FOR BRANCH CIRCUIT WIRING. FITTINGS SHALL BE HEX-NUT, COMPRESSION TYPE, ZINC PLATED, AND U.L. LISTED AS RAINTIGHT. NO CRIMP, SPRING, OR SET-SCREW TYPE FITTINGS WILL BE ACCEPTED. EXPOSED CONDUITS SHALL BE RIGID GALVANIZED STEEL. CONNECTORS AND COUPLINGS SHALL BE STEEL, THREADED TYPE. PAINT EXPOSED CONDUIT, COUPLINGS AND CONNECTORS WITH ZINC PRIMER AND ONE FINISH COAT OF AIR DRIED ENAMEL. CONDUIT USED BELOW GRADE MAY BE SCHEDULE 40 PVC; TRANSITION TO RGS AT ELBOW BEFORE TURNING UP ABOVE GRADE. FURNISH AND INSTALL SLEEVES (GALVANIZED STEEL) FOR ALL CONDUIT PENETRATIONS IN SLAB OR WALLS. MINIMUM CONDUIT SIZE SHALL BE 1/2".

M. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CLEARANCES PRIOR TO INSTALLATION OF EQUIPMENT AND RACEWAYS.

N. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL MATERIALS FOR ELECTRICAL INSTALLATION. ALL MATERIALS SHALL BE WITH U.L. LABELS. ALL WORK SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE FASHION.

O. TYPE MC CABLE MAY BE USED IN CONCEALED LOCATIONS WHERE ALLOWED BY LOCAL CODES AND SHALL BE REFLECTED AS A COST SAVINGS TO THE OWNER. MC CABLE SHALL NOT BE USED TO ENTER PANELBOARDS.

P. PROVIDE GROUNDING FOR ALL EQUIPMENT IN ACCORDANCE WITH ARTICLE 250 OF THE NATIONAL ELECTRICAL CODE.

Q. ALL WORK SHALL HAVE PROPER LABELING. ALL CIRCUITS SHALL BE LABELED AT PANELS AND BOXES AS INDICATED. ALL PANELS AND DISCONNECTS SHALL BE PERMANENTLY MARKED WITH NAME OR EQUIPMENT SERVED. ALL PANELS SHALL BE PROVIDED WITH TYPEWRITTEN PANEL SCHEDULES REFLECTING CHANGES FROM THIS REMODEL PROJECT.

R. PROVIDE NEW CIRCUIT BREAKERS IN EXISTING PANELS AS REQUIRED, TYPE, AIC RATING, AND VOLTAGE RATING SHALL MATCH EXISTING. ALL BREAKERS SHALL BE TYPE HACR BREAKERS.

ELECTRICAL KEYED NOTES:

1. EXISTING EMERGENCY LIGHT FIXTURE OR EXIT SIGN SHALL REMAIN, SUBJECT TO REPAIR OR RECIRCUITING PER "EMERGENCY LIGHTING SCOPE OF WORK" NOTE ON THIS PAGE.
2. EXISTING LIGHT FIXTURE TO REMAIN ON EXISTING CIRCUIT AND CONTROLS.
3. NEW UNISTRUT SUPPORT SYSTEM WITH SPOT LIGHTS. TYPE PER ARCHITECTURAL DRAWINGS. CONNECT TO EXISTING LOCAL PATIO LIGHT CIRCUIT. EXISTING ELECTRICAL CONNECTION MAY BE REUSED WHERE PRACTICAL/APPLICABLE.
4. NEW CEILING FAN. TYPE PER ARCHITECTURAL DRAWINGS. CONNECT TO EXISTING FAN CIRCUIT. VERIFY EXACT REQUIREMENTS.
5. EXISTING CEILING FAN TO REMAIN ON EXISTING CIRCUIT AND CONTROLS.
6. GENERAL CONTRACTOR TO PROVIDE ELECTRICAL CONNECTION (DUPLEX OUTLET)) FOR OPERABLE ROLLER SHADE. CONNECT TO SPARE CIRCUIT IN PANEL 'C1'. PROVIDE NEW 20/1 BREAKER, TYPE VOLTAGE AND AIC RATING TO MATCH EXISTING AS REQUIRED.
7. NEW EXTERIOR PLANTER LIGHT. CONNECT TO EXISTING LOCAL PLANTER LIGHT CIRCUIT. EXISTING ELECTRICAL CONNECTION MAY BE REUSED WHERE PRACTICAL/APPLICABLE. COORDINATE EXACT REQUIREMENTS WITH MANUFACTURER'S INSTRUCTIONS. COORDINATE EXACT LOCATIONS WITH OWNER.
8. EXISTING TV TO REMAIN.
9. NEW TV TO BE MOUNTED TO MATCH EXISTING ADJACENT TV HEIGHT. CONNECT TO NEW CIRCUIT INDICATED. EXISTING ELECTRICAL CONNECTION MAY BE REUSED WHERE PRACTICAL/APPLICABLE.
10. PROVIDE RECEPTACLES, J-BOX WITH CONDUIT TO ABOVE CEILING FOR DATA, AND J-BOX WITH CONDUIT TO ABOVE CEILING FOR PHONE AT POS STATION. CONNECT TO NEW CIRCUIT INDICATED. COORDINATE EXACT LOCATIONS, CIRCUIT, AND REQUIREMENTS PRIOR TO WORK. COORDINATE EXACT MOUNTING LOCATION AND HEIGHT WITH OWNER, ARCHITECTURAL ELEVATIONS, AND MILLWORK PRIOR TO ROUGH-IN.
11. PROVIDE ALL WORK REQUIRED FOR NEW SIGN ROUGH-IN. COORDINATE EXACT REQUIREMENTS AND LOCATION WITH ARCHITECTURAL ELEVATIONS. CONNECT TO CIRCUIT INDICATED. ROUTE THROUGH EXISTING CONTACTOR.

EMERGENCY LIGHTING SCOPE OF WORK

UNLESS NOTED OTHERWISE, ALL EMERGENCY EGRESS LIGHTS AND EXIT SIGNS ARE TO REMAIN IN EXISTING LOCATIONS. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING EMERGENCY EGRESS LIGHTS AND EXIT SIGNS ARE FUNCTIONAL AND SHALL REPAIR OR REPLACE EXISTING FIXTURES AS REQUIRED. RECONNECTION OF DINING AREA NORMAL LIGHTS TO NEW DIMMER PANEL SHALL NOT DISRUPT PROPER OPERATION OF EMERGENCY LIGHTS AND EXIT SIGNS. WHERE APPLICABLE, EMERGENCY LIGHTS AND EXIT SIGNS MAY BE CONNECTED TO LOCKED-ON BREAKER IN NEW DIMMER PANEL AS ALLOWED BY EXCEPTION TO NEC ARTICLE 700.12(F). OTHERWISE, THESE FIXTURES SHALL BE CONNECTED TO UNSWITCHED CONTINUOUSLY HOT CONDUCTOR OF THE LIGHTING CIRCUIT THAT FEEDS THE INDIVIDUAL DIMMER MODULES. FIELD VERIFY EXACT REQUIREMENTS.

LIGHTING DEMOLITION NOTE:

PLANS REFLECT NEW, EXISTING, RELOCATED LIGHTING ONLY. REFER TO ARCHITECTURAL DRAWINGS FOR LIGHTING DEMOLITION LOCATIONS. WHERE APPLICABLE, REMOVE LIGHTS & CONDUIT & CONDUCTORS BACK TO PANEL

GENERAL EXISTING LIGHTING NOTE:

ALL LIGHT FIXTURES ARE EXISTING UNLESS NOTED OTHERWISE. LOCATIONS ARE INDICATED ON THIS PLAN FOR REFERENCE ONLY. FIELD VERIFY ALL LIGHTING FIXTURE LOCATIONS. WHERE APPLICABLE, RECONNECT EXISTING FIXTURES TO NEW CIRCUITS INDICATED.

EXISTING PANEL GENERAL NOTES:

P1. VERIFY ALL CIRCUITS ON EXISTING PANELS. ADJUST CIRCUITING AS REQUIRED TO MEET DESIGN INTENT OF DRAWINGS. FOR ANY VACATED CIRCUITS, REMOVE CONDUIT AND WIRING BACK TO PANEL, TURN BREAKER OFF, AND MARK BREAKER AS 'SPARE'. FOR EXISTING CIRCUITS MISLABELED BUT CONNECTED TO EXISTING LOAD REQUIRED TO REMAIN, FIELD DETERMINE LOAD AND PROVIDE APPROPRIATE DESCRIPTION IN PANEL DIRECTORY.

P2. PROVIDE NEW TYPEWRITTEN PANEL DIRECTORY TO REFLECT NEW CONDITIONS UPON COMPLETION OF WORK DESCRIBED IN THESE DRAWINGS. CLEAN OFF/REMOVE/ERASE ALL HANDWRITTEN LABELING ON PANEL COVER. CIRCUIT DESCRIPTION SHALL EXIST ON PANEL DIRECTORY ONLY.

P3. REUSE EXISTING CIRCUIT BREAKERS WHERE POSSIBLE. PROVIDE NEW BREAKERS AS REQUIRED, TYPE, VOLTAGE RATING, AND AIC RATING TO MATCH EXISTING.

P4. CLEAN PANELBOARD, AND REPAIR OR REPLACE ANY MISSING OR DAMAGED COMPONENTS ON EXISTING PANELBOARDS AND DIMMING EQUIPMENT. FIELD VERIFY AND COORDINATE EXACT REQUIREMENTS WITH OWNER.

P5. EXISTING CIRCUIT DATA SHOWN IS TAKEN FROM EXISTING FACILITY DRAWINGS AND/OR FIELD OBSERVATION. FIELD VERIFY ALL CIRCUITS.

P6. PANEL SCHEDULE INDICATES PROPOSED CONFIGURATION AFTER ALL CHANGES HAVE BEEN COMPLETED.

PANEL SCHEDULE NOTES

- E EXISTING CIRCUIT TO REMAIN
- G GFI CIRCUIT BREAKER
- IG ISOLATED GROUND CIRCUIT
- LC ROUTE CIRCUIT HOMERUN VIA LIGHTING CONTACTOR
- LF PROVIDE PADLOCK ATTACHMENT FOR MAINTENANCE LOCK-OUT OF CIRCUIT BREAKER
- LO PROVIDE LOCK-ON DEVICE FOR CIRCUIT BREAKER
- N NEW OR REVISED LOAD/DESCRIPTION
- P PRE-WIRED INTERNAL CIRCUIT BY SWITCHGEAR MANUFACTURER
- ST SHUNT-TRIP CIRCUIT BREAKER
- SUB SUB-FEED CIRCUIT BREAKER

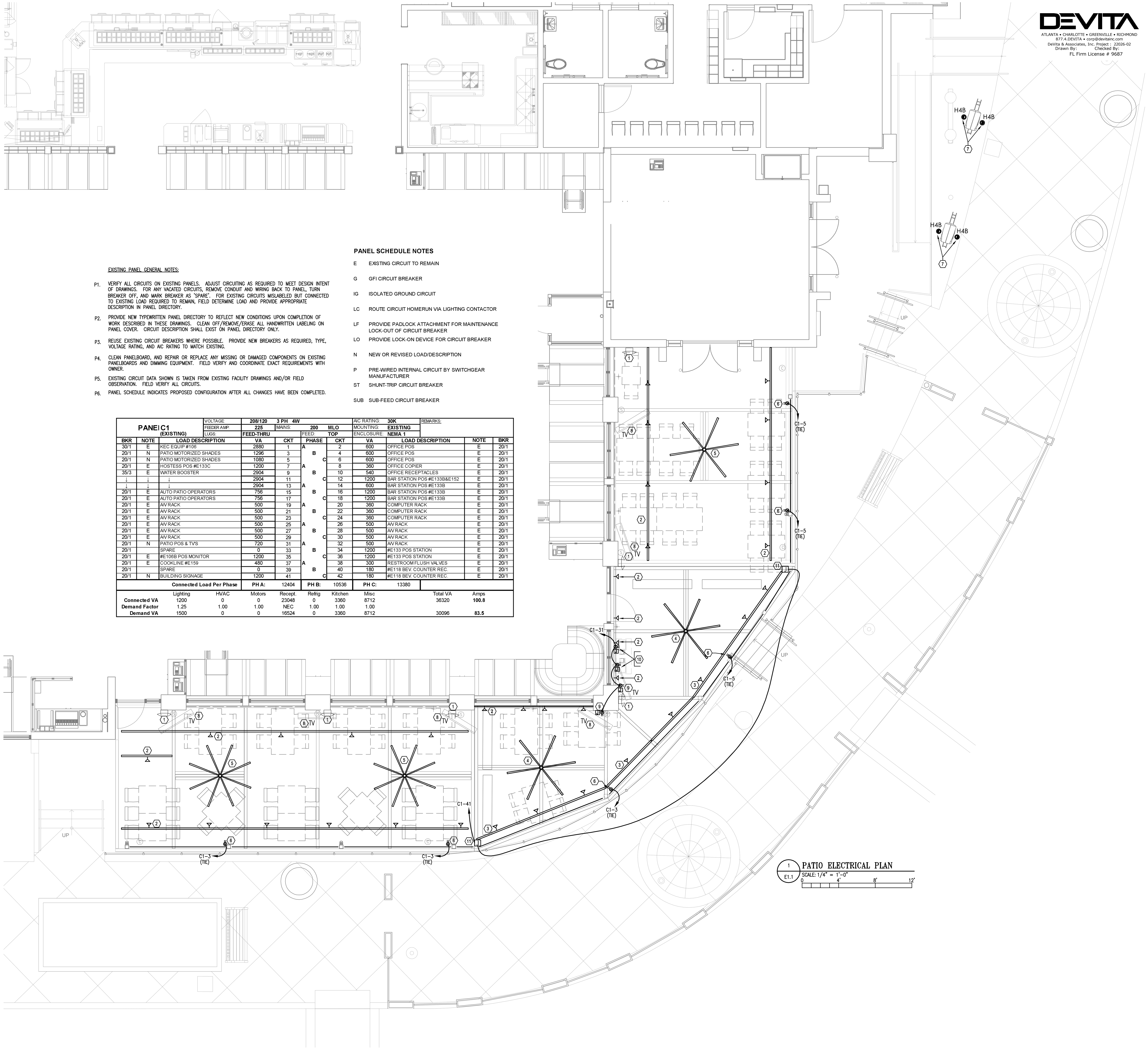
PANEl C1 (EXISTING)

VOLTAGE:	208/120 3 PH 4W	AIC RATING:	30K
FEEDER AMP:	225	MAINS:	200 MLO
LUGS:	FEED-THRU	FEED:	TOP
MOUNTING:	EXISTING	ENCLOSURE:	NEMA 1
REMARKS:			

BKR	NOTE	LOAD DESCRIPTION	VA	CKT	PHASE	CKT	VA	LOAD DESCRIPTION	NOTE	BKR
30/1	E	KEC EQUIP #106	2880	1	A	2	600	OFFICE POS	E	20/1
20/1	N	PATIO MOTORIZED SHADES	1296	3	B	4	600	OFFICE POS	E	20/1
20/1	N	PATIO MOTORIZED SHADES	1080	5	C	6	600	OFFICE POS	E	20/1
20/1	E	HOSTESS POS #E133C	1200	7	A	8	360	OFFICE COPIER	E	20/1
35/3	E	WATER BOOSTER	2904	9	B	10	540	OFFICE RECEPTACLES	E	20/1
↓	↓	↓	2904	11	C	12	1200	BAR STATION POS #E133B&E152	E	20/1
↓	↓	↓	2904	13	A	14	600	BAR STATION POS #E133B	E	20/1
20/1	E	AUTO PATIO OPERATORS	756	15	B	16	1200	BAR STATION POS #E133B	E	20/1
20/1	E	AUTO PATIO OPERATORS	756	17	C	18	1200	BAR STATION POS #E133B	E	20/1
20/1	E	A/V RACK	500	19	A	20	360	COMPUTER RACK	E	20/1
20/1	E	A/V RACK	500	21	B	22	360	COMPUTER RACK	E	20/1
20/1	E	A/V RACK	500	23	C	24	360	COMPUTER RACK	E	20/1
20/1	E	A/V RACK	500	25	A	26	500	A/V RACK	E	20/1
20/1	E	A/V RACK	500	27	B	28	500	A/V RACK	E	20/1
20/1	E	A/V RACK	500	29	C	30	500	A/V RACK	E	20/1
20/1	N	PATIO POS & TV'S	720	31	A	32	500	A/V RACK	E	20/1
20/1		SPARE	0	33	B	34	1200	#E133 POS STATION	E	20/1
20/1	E	#E106B POS MONITOR	1200	35	C	36	1200	#E133 POS STATION	E	20/1
20/1	E	COOKLINE #E159	480	37	A	38	300	RESTROOM FLUSH VALVES	E	20/1
20/1		SPARE	0	39	B	40	180	#E118 BEV. COUNTER REC.	E	20/1
20/1	N	BUILDING SIGNAGE	1200	41	C	42	180	#E118 BEV. COUNTER REC.	E	20/1

Connected Load Per Phase	PH A:	12404	PH B:	10536	PH C:	13380

	Lighting	HVAC	Motors	Recept.	Refrig	Kitchen	Misc	Total VA	Amps
Connected VA	1200	0	0	23048	0	3360	8712	36320	**100.8**
Demand Factor	1.25	1.00	1.00	NEC	1.00	1.00	1.00		
Demand VA	1500	0	0	16524	0	3360	8712	30096	**83.5**



1 PATIO ELECTRICAL PLAN
E1.1 SCALE: 1/4" = 1'-0"

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Orlando, FL

PATIO ELECTRICAL PLAN

E1.1

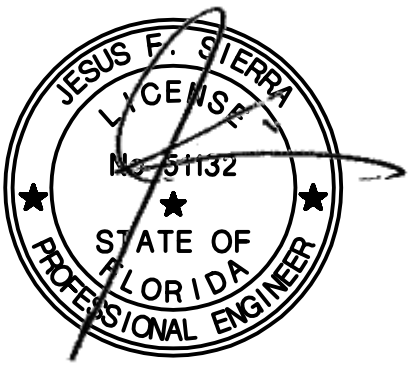
SIERRA
ENGINEERING
G R O U P

STRUCTURAL CALCULATIONS
for

YARDHOUSE

Orlando, Fl

August, 2022



Sealed 08-04-22

▲2600 CENTRAL AVE.,SUITE B, UNION CITY, CALIFORNIA 94587 PH:510.445.0550 FAX:510.445.0440▲

⚠ This is a beta release of the new ATC Hazards by Location website. Please contact us with feedback.

ℹ The ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

Hazards by Location

Search Information

Address: 8367 International Dr, Orlando, FL 32819, USA

Coordinates: 28.4436022, -81.46986530000001

Elevation: 125 ft

Timestamp: 2022-08-02T22:16:53.251Z

Hazard Type: Wind

ASCE 7-16

MRI 10-Year ..... 79 mph

MRI 25-Year ..... 93 mph

MRI 50-Year ..... 104 mph

MRI 100-Year ..... 113 mph

Risk Category I ..... 126 mph

Risk Category II ..... ⚠ 135 mph

You are in a wind-borne debris region if you are also within 1 mile of the coastal mean high water line.

Risk Category III ..... ⚠ 143 mph

If the structure under consideration is a healthcare facility and you are also within 1 mile of the coastal mean high water line, you are in a wind-borne debris region. If other occupancy, use the Risk Category II basic wind speed contours to determine if you are in a wind-borne debris region.

Risk Category IV ..... ⚠ 147 mph

You are in a wind-borne debris region.

ASCE 7-10

MRI 10-Year ..... 79 mph

MRI 25-Year ..... 93 mph

MRI 50-Year ..... 103 mph

MRI 100-Year ..... 113 mph

Risk Category I ..... 126 mph

Risk Category II ..... ⚠ 135 mph

You are in a wind-borne debris region if you are also within 1 mile of the coastal mean high water line.

Risk Category III-IV ... ⚠ 143 mph

If the structure under consideration is a healthcare facility and you are also within 1 mile of the coastal mean high water line, you are in a wind-borne debris region. If other occupancy, use the Risk Category II basic wind speed contours to determine if you are in a wind-borne debris region.

ASCE 7-05

ASCE 7-05 Wind Speed ..... 103 mph

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before

proceeding with design.

Please note that the ATC Hazards by Location website will not be updated to support ASCE 7-22. Find out why.

Disclaimer

Hazard loads are interpolated from data provided in ASCE 7 and rounded up to the nearest whole integer. Per ASCE 7, islands and coastal areas outside the last contour should use the last wind speed contour of the coastal area – in some cases, this website will extrapolate past the last wind speed contour and therefore, provide a wind speed that is slightly higher. NOTE: For queries near wind-borne debris region boundaries, the resulting determination is sensitive to rounding which may affect whether or not it is considered to be within a wind-borne debris region.

Mountainous terrain, gorges, ocean promontories, and special wind regions shall be examined for unusual wind conditions.
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BY SC DATE 08/22 SUBJECT YARDHOUSE SHEET NO. _____ OF _____

CHKD. BY _____ DATE _____ JOB. NO. 22046.0

WIND LOADS:

- ASCE 7-16
- SECTION 30.11 (CANOPIES ON BUILDINGS)

$q_h = 0.00256 K_z K_{zt} K_d K_e V^2$ (26.10)

$= 0.00256 (0.87)(1)(0.85)(1)(135)^2 = 34.5$ PSF

$p = q_h (GC_p)$ (30.11-1)

$= (34.5)(-0.9) =$ **31 PSF**

$h_c/h_e = 16'/20' = 0.8$ (FIG 30.11-1B)

$GC_p = -0.9, +0.9$

BY SC DATE 08/22 SUBJECT YARDHOUSE SHEET NO. ____ OF ____

CHKD. BY ____ DATE ____ JOB. NO. 22046.0

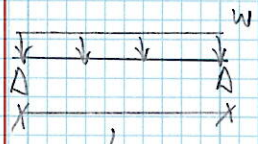
GRAVITY ANALYSIS:

DL = 27 PSF
RLL = 30 PSF
WL = 31 PSF

BM1: (CASE 01)

L = 19'-0"
TRIB = 7'-0"
D/C = 0.38
REACTIONS = 2K DL, 2K RLL, 2K W

USE HSS 14x4x3/16



(CASE 01)

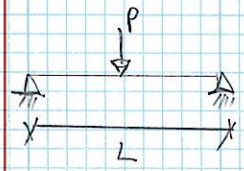
BM2: (CASE 01)

L = 15'
TRIB = 7'-0"
D/C = 0.314
REACTIONS = 1.6K DL, 1.6K RLL, 1.6K W

USE HSS 12x4x3/16

BM3: (CASE 2)

L = 19'-0"
P = BM 02 REACTIONS
D/C = 0.22
REACTIONS = 1.0K DL, 0.8K RLL, 0.8K W



(CASE 02)

BM4: (CASE 2)

L = 7'-0"
P = 500 LBS
D/C = 0.03
REACTIONS = 0.37K DL

USE LL 8x4x7/16

GRAVITY & LATERAL:

C1:

H = 16'-0"

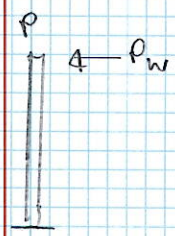
P = BM02, (2) BM03

P_W = (31 PSF WIND)(3'x18')

= 1.6 KIPS

D/C = 0.14

USE HSS 5x5x3/8

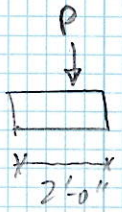


F1:

P = C1 LOADS

(E) 2'-0" WALL FTNG REMAINS ADEQUATE

D/C = 0.92



BY SC DATE 08/22 SUBJECT YARDHOUSE SHEET NO. _____ OF _____

CHKD. BY _____ DATE _____ JOB. NO. 22046.0

CONNECTIONS:

P_1 = 8,000 LBS (SHEAR)

P_2 = 2,000 LBS. (TENSION)

D/C = 0.26

(4) 3/4" ø ANCHORS w/ 6" EMB ARE REQUIRED AT CONC COLUMNS.

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam

Project File: 22046.0.ec6

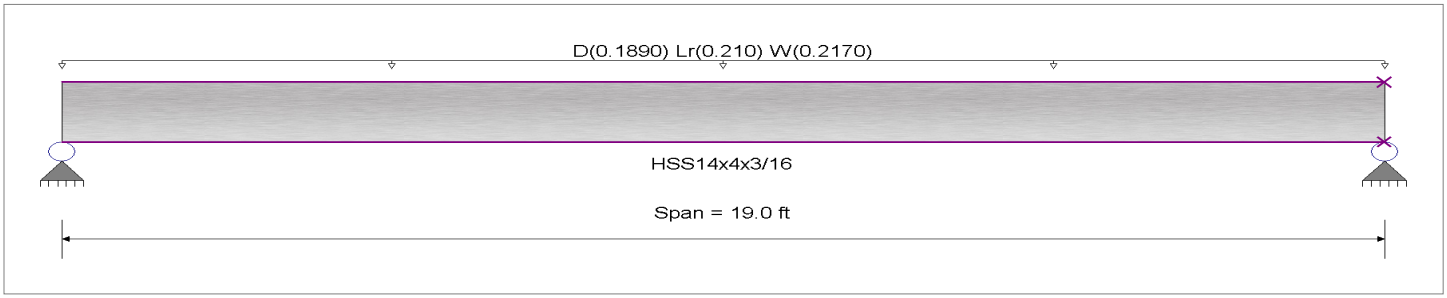
DESCRIPTION: BM1:

CODE REFERENCES

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

Analysis Method :	Allowable Strength Design	Fy : Steel Yield :	46.0 ksi
Beam Bracing :	Beam is Fully Braced against lateral-torsional buckling	E: Modulus :	29,000.0 ksi
Bending Axis :	Major Axis Bending		



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading
Uniform Load : D = 0.0270, Lr = 0.030, W = 0.0310 ksf, Tributary Width = 7.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.380** : 1	Maximum Shear Stress Ratio =	**0.072** : 1
Section used for this span	**HSS14x4x3/16**	Section used for this span	**HSS14x4x3/16**
Ma : Applied	21.043 k-ft	Va : Applied	4.430 k
Mn / Omega : Allowable	55.372 k-ft	Vn/Omega : Allowable	61.494 k
Load Combination	+D+0.750Lr+0.450W	Load Combination	+D+0.750Lr+0.450W
		Location of maximum on span	0.000 ft
Span # where maximum occurs	Span # 1	Span # where maximum occurs	Span # 1

Maximum Deflection

Max Downward Transient Deflection	0.161 in	Ratio =	1,417	>=360	
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360	Span: 1 : W Only
Max Downward Total Deflection	0.346 in	Ratio =	659	>=180	Span: 1 : +D+0.750Lr+0.450W
Max Upward Total Deflection	0.000 in	Ratio =	0	<180	

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment Length	Span #	Max Stress Ratios M	V	Summary of Moment Values Mmax +	Mmax -	Ma Max	Mnx	Mnx/Omega	Cb	Rm	Summary of Shear Values Va Max	VnxVnx/Omega
D Only												
Dsgn. L = 19.00 ft	1	0.172	0.033	9.53		9.53	92.47	55.37	1.00	1.00	2.01	102.69 61.49
+D+Lr												
Dsgn. L = 19.00 ft	1	0.343	0.065	19.01		19.01	92.47	55.37	1.00	1.00	4.00	102.69 61.49
+D+0.750Lr												
Dsgn. L = 19.00 ft	1	0.300	0.057	16.64		16.64	92.47	55.37	1.00	1.00	3.50	102.69 61.49
+D+0.60W												
Dsgn. L = 19.00 ft	1	0.278	0.053	15.40		15.40	92.47	55.37	1.00	1.00	3.24	102.69 61.49
+D+0.750Lr+0.450W												
Dsgn. L = 19.00 ft	1	0.380	0.072	21.04		21.04	92.47	55.37	1.00	1.00	4.43	102.69 61.49
+D+0.450W												
Dsgn. L = 19.00 ft	1	0.252	0.048	13.94		13.94	92.47	55.37	1.00	1.00	2.93	102.69 61.49
+0.60D+0.60W												
Dsgn. L = 19.00 ft	1	0.209	0.040	11.59		11.59	92.47	55.37	1.00	1.00	2.44	102.69 61.49
+0.60D												
Dsgn. L = 19.00 ft	1	0.103	0.020	5.72		5.72	92.47	55.37	1.00	1.00	1.20	102.69 61.49

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+0.750Lr+0.450W	1	0.3457	9.554		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam

Project File: 22046.0.ec6

DESCRIPTION: BM1:

Vertical Reactions

Support notation : Far left is #1 Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	4.430	4.430
Overall MINimum	1.204	1.204
D Only	2.006	2.006
+D+Lr	4.001	4.001
+D+0.750Lr	3.502	3.502
+D+0.60W	3.243	3.243
+D+0.750Lr+0.450W	4.430	4.430
+D+0.450W	2.934	2.934
+0.60D+0.60W	2.441	2.441
+0.60D	1.204	1.204
Lr Only	1.995	1.995
W Only	2.062	2.062

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam

Project File: 22046.0.ec6

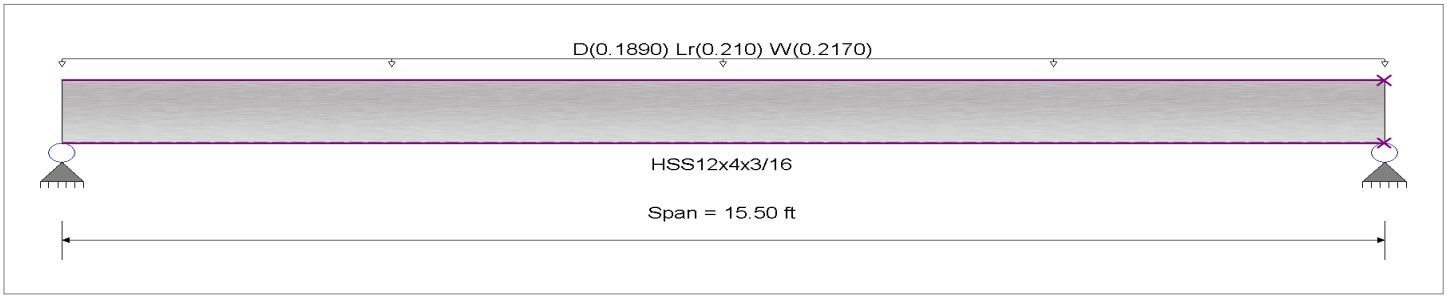
DESCRIPTION: BM2:

CODE REFERENCES

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

Analysis Method :	Allowable Strength Design	Fy : Steel Yield :	46.0 ksi
Beam Bracing :	Beam is Fully Braced against lateral-torsional buckling	E: Modulus :	29,000.0 ksi
Bending Axis :	Major Axis Bending		



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading
Uniform Load : D = 0.0270, Lr = 0.030, W = 0.0310 ksf, Tributary Width = 7.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.314** : 1	Maximum Shear Stress Ratio =	**0.058** : 1
Section used for this span	**HSS12x4x3/16**	Section used for this span	**HSS12x4x3/16**
Ma : Applied	13.928 k-ft	Va : Applied	3.594 k
Mn / Omega : Allowable	44.365 k-ft	Vn/Omega : Allowable	61.804 k
Load Combination	+D+0.750Lr+0.450W	Load Combination	+D+0.750Lr+0.450W
		Location of maximum on span	0.000 ft
Span # where maximum occurs	Span # 1	Span # where maximum occurs	Span # 1

Maximum Deflection

Max Downward Transient Deflection	0.106 in	Ratio =	1,749	>=360	
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360	Span: 1 : W Only
Max Downward Total Deflection	0.227 in	Ratio =	818	>=180	Span: 1 : +D+0.750Lr+0.450W
Max Upward Total Deflection	0.000 in	Ratio =	0	<180	

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment Length	Span #	Max Stress Ratios M	V	Summary of Moment Values Mmax +	Mmax -	Ma Max	Mnx	Mnx/Omega	Cb	Rm	Summary of Shear Values Va Max	VnxVnx/Omega	
D Only													
Dsgn. L = 15.46 ft	1	0.141	0.026	6.27		6.27	74.09	44.37	1.00	1.00	1.62	103.21	61.80
Dsgn. L = 0.04 ft	1	0.002	0.026	0.07		0.07	74.09	44.37	1.00	1.00	1.62	103.21	61.80
+D+Lr													
Dsgn. L = 15.46 ft	1	0.283	0.052	12.57		12.57	74.09	44.37	1.00	1.00	3.24	103.21	61.80
Dsgn. L = 0.04 ft	1	0.003	0.052	0.14		0.14	74.09	44.37	1.00	1.00	3.24	103.21	61.80
+D+0.750Lr													
Dsgn. L = 15.46 ft	1	0.248	0.046	11.00		11.00	74.09	44.37	1.00	1.00	2.84	103.21	61.80
Dsgn. L = 0.04 ft	1	0.003	0.046	0.13		0.13	74.09	44.37	1.00	1.00	2.84	103.21	61.80
+D+0.60W													
Dsgn. L = 15.46 ft	1	0.229	0.042	10.18		10.18	74.09	44.37	1.00	1.00	2.63	103.21	61.80
Dsgn. L = 0.04 ft	1	0.003	0.042	0.12		0.12	74.09	44.37	1.00	1.00	2.63	103.21	61.80
+D+0.750Lr+0.450W													
Dsgn. L = 15.46 ft	1	0.314	0.058	13.93		13.93	74.09	44.37	1.00	1.00	3.59	103.21	61.80
Dsgn. L = 0.04 ft	1	0.004	0.058	0.16		0.16	74.09	44.37	1.00	1.00	3.59	103.21	61.80
+D+0.450W													
Dsgn. L = 15.46 ft	1	0.207	0.038	9.20		9.20	74.09	44.37	1.00	1.00	2.37	103.21	61.80
Dsgn. L = 0.04 ft	1	0.002	0.038	0.10		0.10	74.09	44.37	1.00	1.00	2.37	103.21	61.80
+0.60D+0.60W													
Dsgn. L = 15.46 ft	1	0.173	0.032	7.67		7.67	74.09	44.37	1.00	1.00	1.98	103.21	61.80
Dsgn. L = 0.04 ft	1	0.002	0.032	0.09		0.09	74.09	44.37	1.00	1.00	1.98	103.21	61.80
+0.60D													

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam

Project File: 22046.0.ec6

DESCRIPTION: BM2:

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment Length	Span #	Max Stress Ratios M	V	Summary of Moment Values Mmax +	Mmax -	Ma Max	Mnx	Mnx/Omega	Cb	Rm	Summary of Shear Values Va Max	Vnx	Vnx/Omega
Dsgn. L = 15.46 ft	1	0.085	0.016	3.76		3.76	74.09	44.37	1.00	1.00	0.97	103.21	61.80
Dsgn. L = 0.04 ft	1	0.001	0.016	0.04		0.04	74.09	44.37	1.00	1.00	0.97	103.21	61.80

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+0.750Lr+0.450W	1	0.2273	7.794		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1 Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	3.594	3.594
Overall MINimum	0.970	0.970
D Only	1.617	1.617
+D+Lr	3.244	3.244
+D+0.750Lr	2.838	2.838
+D+0.60W	2.626	2.626
+D+0.750Lr+0.450W	3.594	3.594
+D+0.450W	2.374	2.374
+0.60D+0.60W	1.979	1.979
+0.60D	0.970	0.970
Lr Only	1.628	1.628
W Only	1.682	1.682

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam

Project File: 22046.0.ec6

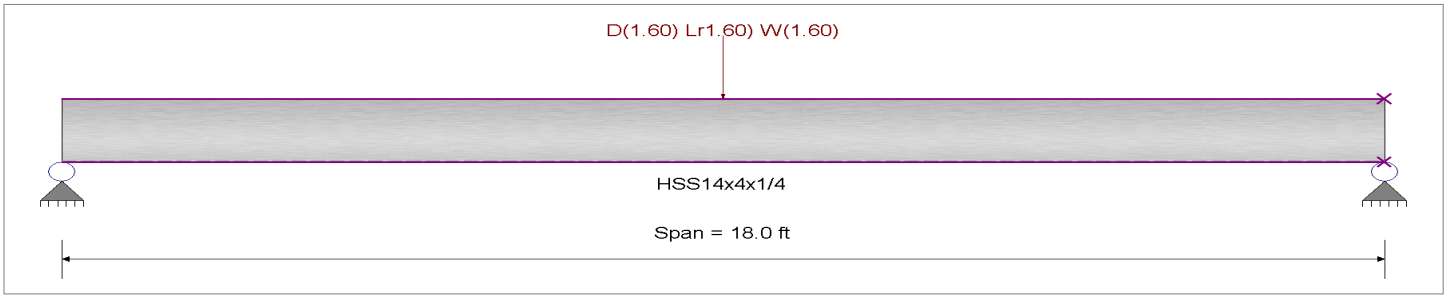
DESCRIPTION: BM3:

CODE REFERENCES

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

Analysis Method :	Allowable Strength Design	Fy : Steel Yield :	46.0 ksi
Beam Bracing :	Beam is Fully Braced against lateral-torsional buckling	E: Modulus :	29,000.0 ksi
Bending Axis :	Major Axis Bending		



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading
Load(s) for Span Number 1
Point Load : D = 1.60, Lr = 1.60, W = 1.60 k @ 9.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.223** : 1	Maximum Shear Stress Ratio =	**0.020** : 1
Section used for this span	**HSS14x4x1/4**	Section used for this span	**HSS14x4x1/4**
Ma : Applied	17.024 k-ft	Va : Applied	2.023 k
Mn / Omega : Allowable	76.208 k-ft	Vn/Omega : Allowable	102.438 k
Load Combination	+D+0.750Lr+0.450W	Load Combination	+D+0.750Lr+0.450W
		Location of maximum on span	0.000 ft
Span # where maximum occurs	Span # 1	Span # where maximum occurs	Span # 1

Maximum Deflection

Max Downward Transient Deflection	0.065 in	Ratio =	3,305	>=360	
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360	Span: 1 : Lr Only
Max Downward Total Deflection	0.157 in	Ratio =	1374	>=180	Span: 1 : +D+0.750Lr+0.450W
Max Upward Total Deflection	0.000 in	Ratio =	0	<180	

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment Length	Span #	Max Stress Ratios M	V	Summary of Moment Values Mmax +	Mmax -	Ma Max	Mnx	Mnx/Omega	Cb	Rm	Summary of Shear Values Va Max	Vnx	Vnx/Omega
D Only													
Dsgn. L = 18.00 ft	1	0.110	0.010	8.38		8.38	127.27	76.21	1.00	1.00	1.06	171.07	102.44
+D+Lr													
Dsgn. L = 18.00 ft	1	0.204	0.018	15.58		15.58	127.27	76.21	1.00	1.00	1.86	171.07	102.44
+D+0.750Lr													
Dsgn. L = 18.00 ft	1	0.181	0.016	13.78		13.78	127.27	76.21	1.00	1.00	1.66	171.07	102.44
+D+0.60W													
Dsgn. L = 18.00 ft	1	0.167	0.015	12.70		12.70	127.27	76.21	1.00	1.00	1.54	171.07	102.44
+D+0.750Lr+0.450W													
Dsgn. L = 18.00 ft	1	0.223	0.020	17.02		17.02	127.27	76.21	1.00	1.00	2.02	171.07	102.44
+D+0.450W													
Dsgn. L = 18.00 ft	1	0.153	0.014	11.62		11.62	127.27	76.21	1.00	1.00	1.42	171.07	102.44
+0.60D+0.60W													
Dsgn. L = 18.00 ft	1	0.123	0.011	9.35		9.35	127.27	76.21	1.00	1.00	1.12	171.07	102.44
+0.60D													
Dsgn. L = 18.00 ft	1	0.066	0.006	5.03		5.03	127.27	76.21	1.00	1.00	0.64	171.07	102.44

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+0.750Lr+0.450W	1	0.1572	9.051		0.0000	0.000

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam

Project File: 22046.0.ec6

DESCRIPTION: BM3:

Vertical Reactions

Support notation : Far left is #1 Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	2.023	2.023
Overall MINimum	0.638	0.638
D Only	1.063	1.063
+D+Lr	1.863	1.863
+D+0.750Lr	1.663	1.663
+D+0.60W	1.543	1.543
+D+0.750Lr+0.450W	2.023	2.023
+D+0.450W	1.423	1.423
+0.60D+0.60W	1.118	1.118
+0.60D	0.638	0.638
Lr Only	0.800	0.800
W Only	0.800	0.800

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam

Project File: 22046.0.ec6

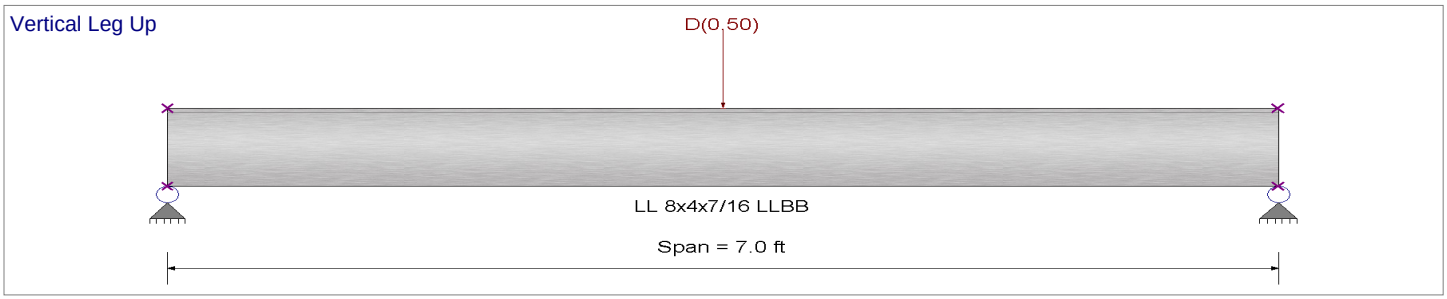
DESCRIPTION: BM4:

CODE REFERENCES

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

Analysis Method :	Allowable Strength Design	Fy : Steel Yield :	46.0 ksi
Beam Bracing :	Completely Unbraced	E: Modulus :	29,000.0 ksi
Bending Axis :	Major Axis Bending		



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading
Load(s) for Span Number 1
Point Load : D = 0.50 k @ 3.50 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	**0.032** : 1	Maximum Shear Stress Ratio =	**0.003** : 1
Section used for this span	**LL 8x4x7/16 LLBB**	Section used for this span	**LL 8x4x7/16 LLBB**
Ma : Applied	1.086 k-ft	Va : Applied	0.3704 k
Mn / Omega : Allowable	34.331 k-ft	Vn/Omega : Allowable	115.821 k
Load Combination	D Only	Load Combination	D Only
		Location of maximum on span	0.000 ft
Span # where maximum occurs	Span # 1	Span # where maximum occurs	Span # 1

Maximum Deflection

Max Downward Transient Deflection	0.000 in	Ratio =	0	<360
Max Upward Transient Deflection	0.000 in	Ratio =	0	<360
Max Downward Total Deflection	0.004 in	Ratio =	20654	>=180 Span: 1 : D Only
Max Upward Total Deflection	0.000 in	Ratio =	0	<180

Maximum Forces & Stresses for Load Combinations

Load Combination / Segment Length	Span #	Max Stress Ratios M	V	Summary of Moment Values Mmax +	Mmax -	Ma Max	Mnx	Mnx/Omega	Cb	Rm	Summary of Shear Values Va Max	VnxVnx/Omega
D Only												
Dsgn. L = 7.00 ft	1	0.032	0.003	1.09		1.09	57.33	34.33	1.28	1.00	0.37	193.42 115.82
+0.60D												
Dsgn. L = 7.00 ft	1	0.019	0.002	0.65		0.65	57.33	34.33	1.28	1.00	0.22	193.42 115.82

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
D Only	1	0.0041	3.520		0.0000	0.000

Vertical Reactions

Support notation : Far left is #: Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	0.370	0.370
Overall MINimum	0.222	0.222
D Only	0.370	0.370
+0.60D	0.222	0.222

Project Title:
Engineer:
Project ID:
Project Descr:

Steel Column

Project File: 22046.0.ec6

DESCRIPTION: C1:

Code References

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Steel Section Name : **HSS5x5x3/8**

Analysis Method :	Allowable Strength
Steel Stress Grade	
Fy : Steel Yield	46 ksi
E : Elastic Bending Modulus	29,000.0 ksi

Overall Column Height	16 ft
Top & Bottom Fixity	Top & Bottom Pinned

Brace condition for deflection (buckling) along columns :
X-X (width) axis :
Fully braced against buckling ABOUT Y-Y Axis
Y-Y (depth) axis :
Fully braced against buckling ABOUT X-X Axis

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 357.920 lbs * Dead Load Factor
AXIAL LOADS . . .
BM2: Axial Load at 16.0 ft, Xecc = 5.0 in, D = 1.60, LR = 1.60, W = 1.60 k
BM3: Axial Load at 16.0 ft, Xecc = 4.50 in, D = 1.0, LR = 0.80, W = 0.80 k
BM3: Axial Load at 16.0 ft, Xecc = 4.50 in, D = 1.0, LR = 0.80, W = 0.80 k
BENDING LOADS . . .
Lat. Point Load at 0.0 ft creating Mx-x, W = 1.70 k

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio = **0.1436** : 1

Load Combination	+D+0.750Lr+0.450W	
Location of max.above base	16.0	ft
At maximum location values are . . .		
Pa : Axial	7.798	k
Pn / Omega : Allowabl	170.228	k
Ma-x : Applied	0.0	k-ft
Mn-x / Omega : Allowable	24.331	k-ft
Ma-y : Applied	-2.937	k-ft
Mn-y / Omega : Allowable	24.331	k-ft

PASS Maximum Shear Stress Rati **0.004025** : 1

Load Combination	+D+0.750Lr+0.450W	
Location of max.above base	0.0	ft
At maximum location values are . . .		
Va : Applied	0.1835	k
Vn / Omega : Allowable	45.601	k

Maximum Load Reactions . .

Top along X-X	0.1835 k
Bottom along X-X	0.1835 k
Top along Y-Y	0.0 k
Bottom along Y-Y	0.0 k

Maximum Load Deflections . . .

Along Y-Y 0.0 in at 0.0 ft above base
for load combination :

Along X-X -0.1336 in at 9.342 ft above base
for load combination : +D+0.750Lr+0.450W

Load Combination Results

Load Combination	Maximum Axial + Bending Stress Ratios: Stress Ratio	Status	Location	Cbx	Cby	KxLx/Ry	KyLy/Rx	Maximum Shear Ratios: Stress Ratio	Status	Location
D Only	0.070	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.002	PASS	0.00 ft
+D+Lr	0.131	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.004	PASS	0.00 ft
+D+0.750Lr	0.116	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.003	PASS	0.00 ft
+D+0.60W	0.107	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.003	PASS	0.00 ft
+D+0.750Lr+0.450W	0.144	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.004	PASS	0.00 ft
+D+0.450W	0.098	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.003	PASS	0.00 ft
+0.60D+0.60W	0.079	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.002	PASS	0.00 ft
+0.60D	0.042	PASS	16.00 ft	1.00	1.00	0.00	0.00	0.001	PASS	0.00 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction k @ Base	@ Top	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
D Only	3.958	0.089	0.089						
+D+Lr	7.158	0.168	0.168						

Project Title:
Engineer:
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Steel Column

Project File: 22046.0.ec6

DESCRIPTION: C1:

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	Axial Reaction @ Base	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
+D+0.750Lr	6.358	0.148	0.148							
+D+0.60W	5.878	0.136	0.136							
+D+0.750Lr+0.450W	7.798	0.184	0.184							
+D+0.450W	5.398	0.124	0.124							
+0.60D+0.60W	4.295	0.101	0.101							
+0.60D	2.375	0.053	0.053							
Lr Only	3.200	0.079	0.079							
W Only	3.200	0.079	0.079							

Extreme Reactions

Item	Extreme Value	Axial Reaction @ Base	X-X Axis Reaction @ Base	@ Top	k	Y-Y Axis Reaction @ Base	@ Top	Mx - End Moments k-ft @ Base	@ Top	My - End Moments @ Base	@ Top
Axial @ Base	Maximum	7.798	0.184	0.184							-2.937
"	Minimum	2.375	0.053	0.053							-0.850
Reaction, X-X Axis Base	Maximum	7.798	0.184	0.184							-2.937
"	Minimum	2.375	0.053	0.053							-0.850
Reaction, Y-Y Axis Base	Maximum	3.958	0.089	0.089							-1.417
"	Minimum	3.958	0.089	0.089							-1.417
Reaction, X-X Axis Top	Maximum	7.798	0.184	0.184							-2.937
"	Minimum	2.375	0.053	0.053							-0.850
Reaction, Y-Y Axis Top	Maximum	3.200	0.079	0.079							-1.267
"	Minimum	3.958	0.089	0.089							-1.417
Moment, X-X Axis Base	Maximum	3.958		0.089							-1.417
"	Minimum	3.958		0.089							-1.417
Moment, Y-Y Axis Base	Maximum	3.958	0.089	0.089					-1.417		
"	Minimum	3.958	0.089	0.089					-1.417		
Moment, X-X Axis Top	Maximum	3.958	0.089	0.089							-1.417
"	Minimum	3.958	0.089	0.089							-1.417
Moment, Y-Y Axis Top	Maximum	2.375	0.053	0.053							-0.850
"	Minimum	7.798	0.184	0.184							-2.937

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	-0.0644 in	9.342 ft	0.000 in	0.000 ft
+D+Lr	-0.1221 in	9.342 ft	0.000 in	0.000 ft
+D+0.750Lr	-0.1077 in	9.342 ft	0.000 in	0.000 ft
+D+0.60W	-0.0990 in	9.342 ft	0.000 in	0.000 ft
+D+0.750Lr+0.450W	-0.1336 in	9.342 ft	0.000 in	0.000 ft
+D+0.450W	-0.0904 in	9.342 ft	0.000 in	0.000 ft
+0.60D+0.60W	-0.0732 in	9.342 ft	0.000 in	0.000 ft
+0.60D	-0.0387 in	9.342 ft	0.000 in	0.000 ft
Lr Only	-0.0576 in	9.342 ft	0.000 in	0.000 ft
W Only	-0.0576 in	9.342 ft	0.000 in	0.000 ft

Steel Section Properties : HSS5x5x3/8

Steel Section Properties : HSS5x5x3/8

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Steel Column

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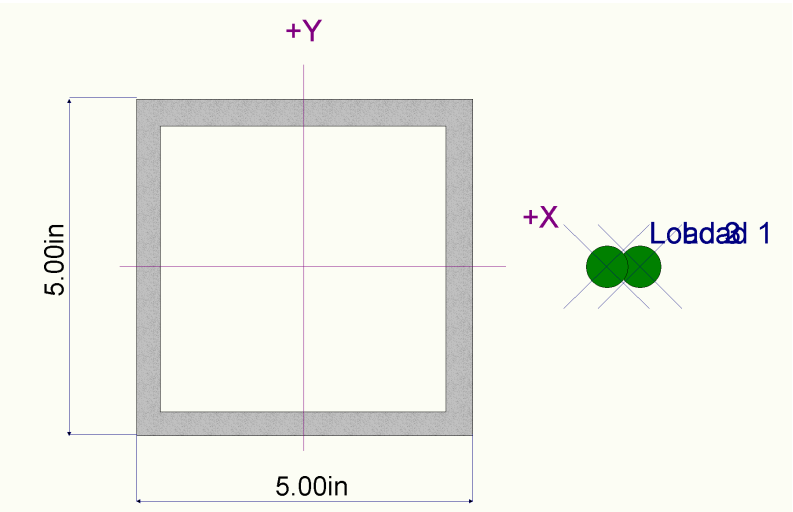
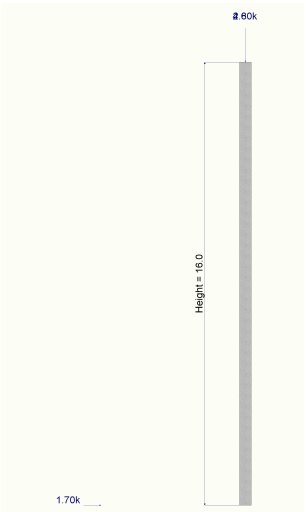
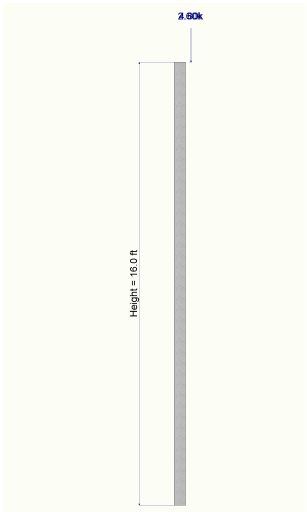
DESCRIPTION: C1:

Depth	=	5.000	in	I xx	=	21.70	in^4	J	=	36.100	in^4
Design Thick	=	0.349	in	S xx	=	8.68	in^3				
Width	=	5.000	in	R xx	=	1.870	in				
Wall Thick	=	0.375	in	Zx	=	10.600	in^3				
Area	=	6.180	in^2	I yy	=	21.700	in^4	C	=	14.900	in^3
Weight	=	22.370	plf	S yy	=	8.680	in^3				
				R yy	=	1.870	in				
Ycg	=	0.000	in								

Sketches







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General Footing

Project File: 22046.0.ec6

DESCRIPTION: F1: Column support

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.0	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

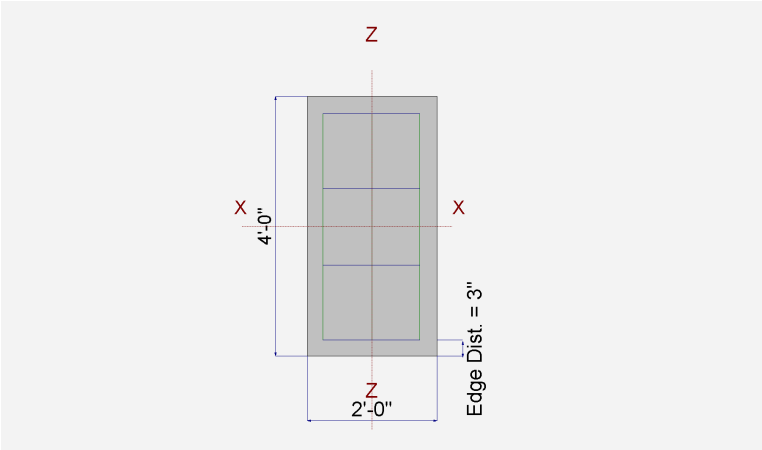
Footing base depth below soil surface	=	ft
Allow press. increase per foot of depth	=	ksf
when footing base is below	=	ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=	ksf
when max. length or width is greater than	=	ft

Dimensions

Width parallel to X-X Axis	=	2.0 ft
Length parallel to Z-Z Axis	=	4.0 ft
Footing Thickness	=	12.0 in
Load location offset from footing center...		
ex : Prll to X-X Axis	=	3 in
	=	in
Pedestal dimensions...		
px : parallel to X-X Axis	=	in
pz : parallel to Z-Z Axis	=	in
Height	=	in
Rebar Centerline to Edge of Concrete...		
at Bottom of footing	=	3.0 in



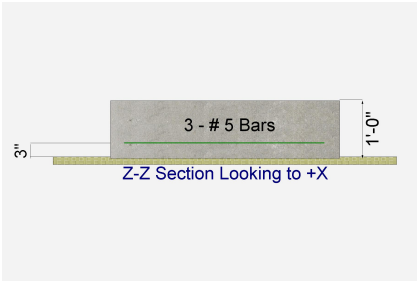
Reinforcing

Bars parallel to X-X Axis			
Number of Bars	=		4
Reinforcing Bar Size	=	#	5
Bars parallel to Z-Z Axis			
Number of Bars	=		3.0
Reinforcing Bar Size	=	#	5

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation

	Bars along X-X Axis
# Bars required within zone	66.7 %
# Bars required on each side of zone	33.3 %



Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	4.0	3.20			3.20			k
OB : Overburden	=								ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k

Project Title:
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General Footing

Project File: 22046.0.ec6

DESCRIPTION: F1: Column support

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9265	Soil Bearing	1.853 ksf	2.0 ksf	+D+0.750Lr+0.450W about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.05216	Z Flexure (+X)	0.6327 k-ft/ft	12.131 k-ft/ft	+1.20D+1.60Lr+0.50W
PASS	0.05216	Z Flexure (-X)	0.6328 k-ft/ft	12.131 k-ft/ft	+1.20D+1.60Lr+0.50W
PASS	0.1611	X Flexure (+Z)	2.880 k-ft/ft	17.879 k-ft/ft	+1.20D+1.60Lr+0.50W
PASS	0.1611	X Flexure (-Z)	2.880 k-ft/ft	17.879 k-ft/ft	+1.20D+1.60Lr+0.50W
PASS	n/a	1-way Shear (+X)	0.0 psi	82.158 psi	n/a
PASS	0.03550	1-way Shear (-X)	2.917 psi	82.158 psi	+1.20D+1.60Lr+0.50W
PASS	0.2012	1-way Shear (+Z)	16.533 psi	82.158 psi	+1.20D+1.60Lr+0.50W
PASS	0.2012	1-way Shear (-Z)	16.533 psi	82.158 psi	+1.20D+1.60Lr+0.50W
PASS	0.2020	2-way Punching	33.188 psi	164.317 psi	+1.20D+1.60Lr+0.50W

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.0	n/a	0.0	0.6450	0.6450	n/a	n/a	0.323
X-X, +D+Lr	2.0	n/a	0.0	1.045	1.045	n/a	n/a	0.523
X-X, +D+0.750Lr	2.0	n/a	0.0	0.9450	0.9450	n/a	n/a	0.473
X-X, +D+0.60W	2.0	n/a	0.0	0.8850	0.8850	n/a	n/a	0.443
X-X, +D+0.750Lr+0.450W	2.0	n/a	0.0	1.125	1.125	n/a	n/a	0.563
X-X, +D+0.450W	2.0	n/a	0.0	0.8250	0.8250	n/a	n/a	0.413
X-X, +0.60D+0.60W	2.0	n/a	0.0	0.6270	0.6270	n/a	n/a	0.314
X-X, +0.60D	2.0	n/a	0.0	0.3870	0.3870	n/a	n/a	0.194
Z-Z, D Only	2.0	2.326	n/a	n/a	n/a	0.2738	1.016	0.508
Z-Z, +D+Lr	2.0	2.584	n/a	n/a	n/a	0.3768	1.713	0.857
Z-Z, +D+0.750Lr	2.0	2.540	n/a	n/a	n/a	0.3510	1.539	0.770
Z-Z, +D+0.60W	2.0	2.508	n/a	n/a	n/a	0.3356	1.434	0.717
Z-Z, +D+0.750Lr+0.450W	2.0	2.613	n/a	n/a	n/a	0.3974	1.853	0.927
Z-Z, +D+0.450W	2.0	2.473	n/a	n/a	n/a	0.3201	1.330	0.665
Z-Z, +0.60D+0.60W	2.0	2.584	n/a	n/a	n/a	0.2261	1.028	0.514
Z-Z, +0.60D	2.0	2.326	n/a	n/a	n/a	0.1643	0.6098	0.305

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

Sliding Stability

All units k

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	1.40	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.40D	1.40	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+0.50Lr	1.60	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+0.50Lr	1.60	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D	1.20	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D	1.20	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+1.60Lr	2.480	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+1.60Lr	2.480	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+1.60Lr+0.50W	2.880	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK

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General Footing

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DESCRIPTION: F1: Column support

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.20D+1.60Lr+0.50W	2.880	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+0.50W	1.60	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+0.50W	1.60	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+0.50Lr+W	2.40	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+0.50Lr+W	2.40	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+W	2.0	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +1.20D+W	2.0	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +0.90D+W	1.70	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +0.90D+W	1.70	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +0.90D	0.90	+Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
X-X, +0.90D	0.90	-Z	Bottom	0.2592	AsMin	0.4650	17.879	OK
Z-Z, +1.40D	0.3076	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.40D	0.3076	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+0.50Lr	0.3515	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+0.50Lr	0.3515	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D	0.2637	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D	0.2636	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+1.60Lr	0.5449	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+1.60Lr	0.5448	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+1.60Lr+0.50W	0.6328	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+1.60Lr+0.50W	0.6327	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+0.50W	0.3515	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+0.50W	0.3515	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+0.50Lr+W	0.5273	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+0.50Lr+W	0.5273	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+W	0.4394	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +1.20D+W	0.4394	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +0.90D+W	0.3735	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +0.90D+W	0.3735	+X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +0.90D	0.1977	-X	Bottom	0.2592	AsMin	0.310	12.131	OK
Z-Z, +0.90D	0.1977	+X	Bottom	0.2592	AsMin	0.310	12.131	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	1.42 psi	0.00 psi	8.04 psi	8.04 psi	8.04 psi	82.16 psi	0.10	OK
+1.20D+0.50Lr	1.62 psi	0.00 psi	9.19 psi	9.19 psi	9.19 psi	82.16 psi	0.11	OK
+1.20D	1.22 psi	0.00 psi	6.89 psi	6.89 psi	6.89 psi	82.16 psi	0.08	OK
+1.20D+1.60Lr	2.51 psi	0.00 psi	14.24 psi	14.24 psi	14.24 psi	82.16 psi	0.17	OK
+1.20D+1.60Lr+0.50W	2.92 psi	0.00 psi	16.53 psi	16.53 psi	16.53 psi	82.16 psi	0.20	OK
+1.20D+0.50W	1.62 psi	0.00 psi	9.19 psi	9.19 psi	9.19 psi	82.16 psi	0.11	OK
+1.20D+0.50Lr+W	2.43 psi	0.00 psi	13.78 psi	13.78 psi	13.78 psi	82.16 psi	0.17	OK
+1.20D+W	2.03 psi	0.00 psi	11.48 psi	11.48 psi	11.48 psi	82.16 psi	0.14	OK
+0.90D+W	1.72 psi	0.00 psi	9.76 psi	9.76 psi	9.76 psi	82.16 psi	0.12	OK
+0.90D	0.91 psi	0.00 psi	5.17 psi	5.17 psi	5.17 psi	82.16 psi	0.06	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	16.13 psi	164.32psi	0.09818	OK
+1.20D+0.50Lr	18.44 psi	164.32psi	0.1122	OK
+1.20D	13.83 psi	164.32psi	0.08416	OK
+1.20D+1.60Lr	28.58 psi	164.32psi	0.1739	OK
+1.20D+1.60Lr+0.50W	33.19 psi	164.32psi	0.202	OK
+1.20D+0.50W	18.44 psi	164.32psi	0.1122	OK
+1.20D+0.50Lr+W	27.66 psi	164.32psi	0.1683	OK
+1.20D+W	23.05 psi	164.32psi	0.1403	OK
+0.90D+W	19.59 psi	164.32psi	0.1192	OK
+0.90D	10.37 psi	164.32psi	0.06312	OK

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Phone:			
E-mail:			

1.Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General
Design method:ACI 318-14
Units: Imperial units

Anchor Information:
Anchor type: Bonded anchor
Material: F1554 Grade 36
Diameter (inch): 0.750
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: ICC-ES ESR-4057
Anchor category: -
Anchor ductility: Yes
h_{min} (inch): 7.75
c_{ac} (inch): 10.89
C_{min} (inch): 1.75
S_{min} (inch): 3.00

Base Material
Concrete: Normal-weight
Concrete thickness, h (inch): 12.00
State: Cracked
Compressive strength, f'_c (psi): 3000
$\Psi_{c,V}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Continuous
Temperature range, Short/Long: 150/110°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate
Length x Width x Thickness (inch): 7.00 x 20.00 x 0.50

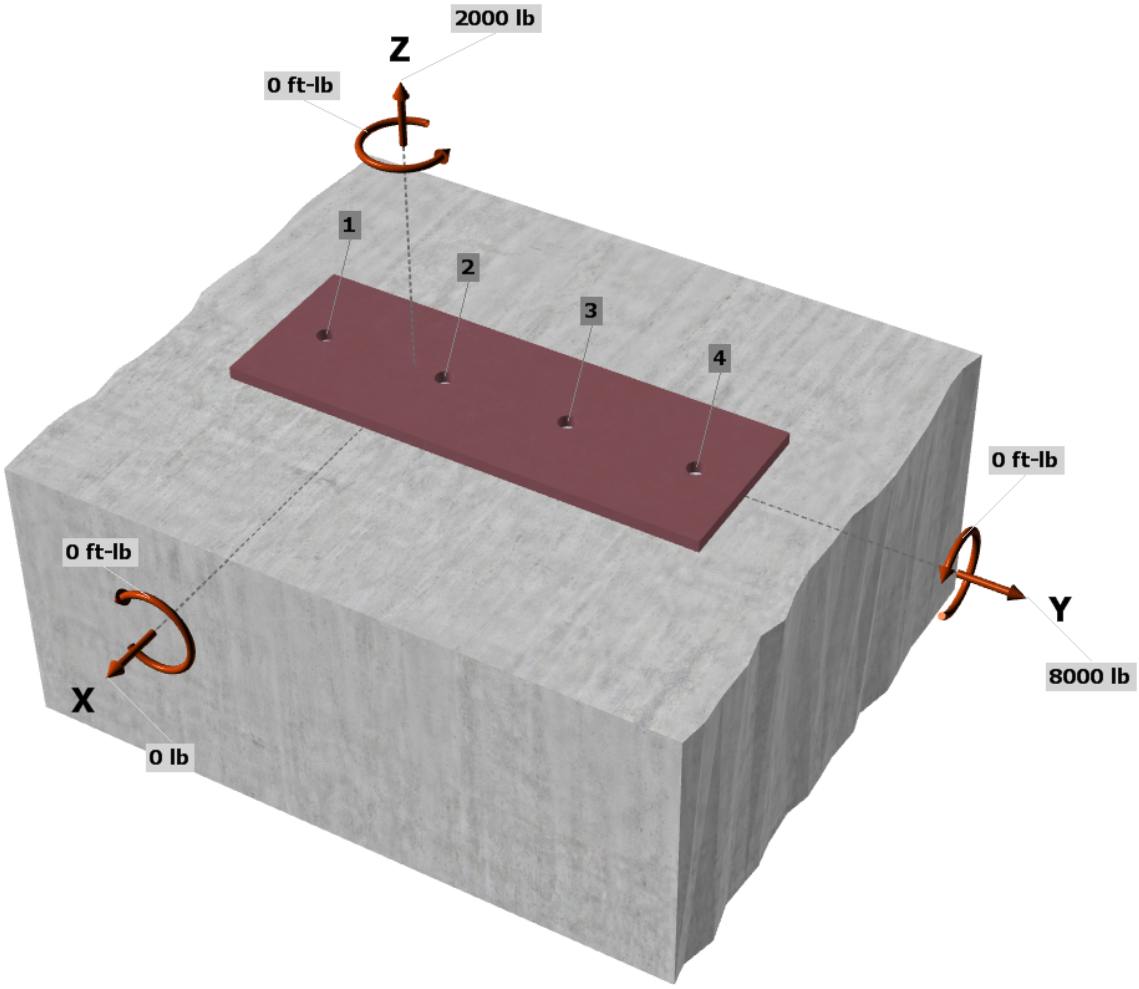
Recommended Anchor
Anchor Name: SET-3G - SET-3G w/ 3/4"Ø F1554 Gr. 36
Code Report: ICC-ES ESR-4057

Load and Geometry
Load factor source: ACI 318 Section 5.3
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

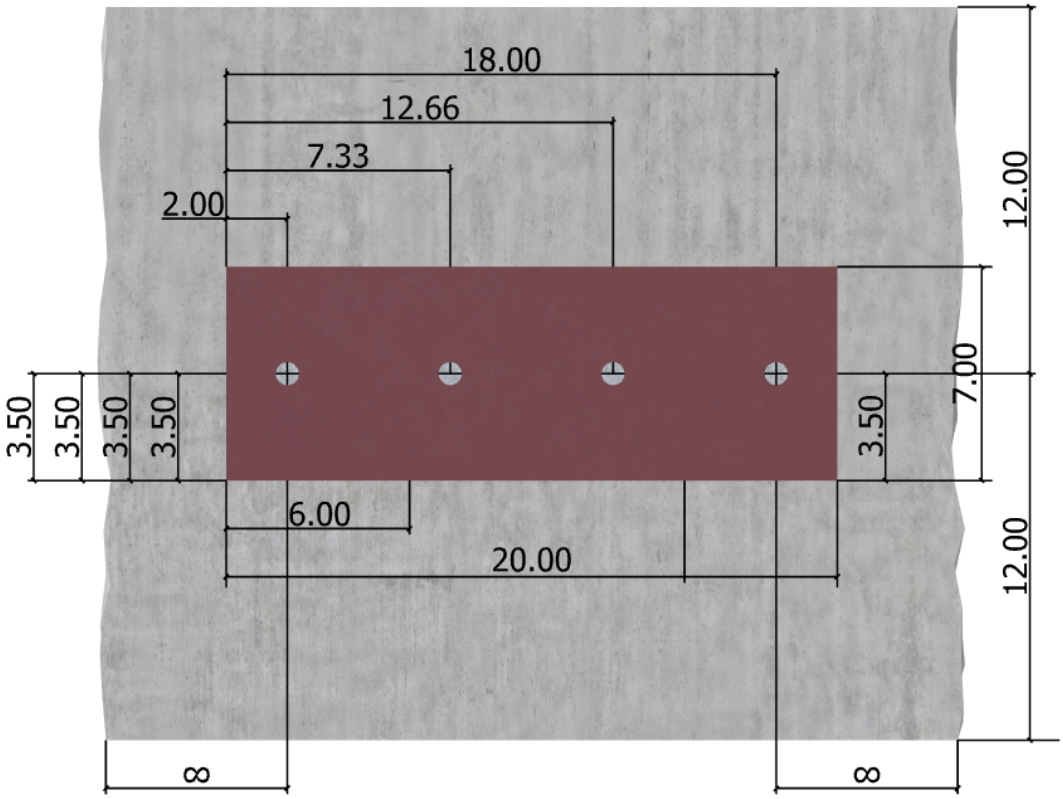
Strength level loads:

N_{ua} [lb]: 2000
V_{uax} [lb]: 0
V_{uay} [lb]: 8000
M_{ux} [ft-lb]: 0
M_{uy} [ft-lb]: 0
M_{uz} [ft-lb]: 0

<Figure 1>



<Figure 2>

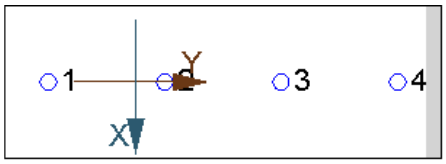


3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2+(V_{uay})^2}$ (lb)
1	941.2	0.0	2000.0	2000.0
2	651.7	0.0	2000.0	2000.0
3	362.3	0.0	2000.0	2000.0
4	72.3	0.0	2000.0	2000.0
Sum	2027.5	0.0	8000.0	8000.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 12
Resultant tension force (lb): 2027
Resultant compression force (lb): 27
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 3.81
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
19370	0.75	14528

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$N_b = k_c \lambda_a \sqrt{f'_c} h_{ef}^{1.5}$ (Eq. 17.4.2.2a)

k_c	λ_a	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	3000	6.000	13685

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b$ (Sec. 17.3.1 & Eq. 17.4.2.1b)

A_{Nc} (in^2)	A_{Nco} (in^2)	$c_{a,min}$ (in)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
612.00	324.00	12.00	0.703	1.000	1.00	1.000	13685	0.65	11805

6. Adhesive Strength of Anchor in Tension (Sec. 17.4.5)

$\tau_{k,cr} = \tau_{k,cr} f_{short\text{-}term} K_{sat} (f'_c / 2{,}500)^n$

$\tau_{k,cr}$ (psi)	$f_{short\text{-}term}$	K_{sat}	f'_c (psi)	n	$\tau_{k,cr}$ (psi)
1310	1.00	1.00	3000	0.24	1369

$N_{ba} = \lambda_a \tau_{cr} \pi d_a h_{ef}$ (Eq. 17.4.5.2)

λ_a	τ_{cr} (psi)	d_a (in)	h_{ef} (in)	N_{ba} (lb)
1.00	1369	0.75	6.000	19348

$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ec,Na} \Psi_{ed,Na} \Psi_{cp,Na} N_{ba}$ (Sec. 17.3.1 & Eq. 17.4.5.1b)

A_{Na} (in^2)	A_{Na0} (in^2)	c_{Na} (in)	$c_{a,min}$ (in)	$\Psi_{ec,Na}$	$\Psi_{ed,Na}$	$\Psi_{cp,Na}$	N_{ba}(lb)	ϕ	ϕN_{ag} (lb)
750.93	422.18	10.27	12.00	0.730	1.000	1.000	19348	0.65	16319

8. Steel Strength of Anchor in Shear (Sec. 17.5.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
11625	1.0	0.65	7556

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)

Shear parallel to edge in x-direction:

$V_{by} = \min|7(l_e / d_a)^{0.2}\sqrt{d_a}\lambda_a\sqrt{f'_c}c_{a1}^{1.5};\ 9\lambda_a\sqrt{f'_c}c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
6.00	0.750	1.00	3000	12.00	20492

$\phi V_{cbgx} = \phi\ (2)(A_{Vc} / A_{Vco})\Psi_{ec,V}\Psi_{ed,V}\Psi_{c,V}\Psi_{h,V}V_{by}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

A_{Vc} (in^2)	A_{Vco} (in^2)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
624.00	648.00	1.000	1.000	1.000	1.225	20492	0.70	33834

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)

$\phi V_{cpg} = \phi \min|k_{cp}N_{ag}\ ;\ k_{cp}N_{cbg}| = \phi \min|k_{cp}(A_{Na} / A_{Na0})\Psi_{ec,Na}\Psi_{ed,Na}\Psi_{cp,Na}N_{ba}\ ;\ k_{cp}(A_{Nc} / A_{Nco})\Psi_{ec,N}\Psi_{ed,N}\Psi_{c,N}\Psi_{cp,N}N_b|$ (Sec. 17.3.1 & Eq. 17.5.3.1b)

k_{cp}	A_{Na} (in^2)	A_{Na0} (in^2)	$\Psi_{ed,Na}$	$\Psi_{ec,Na}$	$\Psi_{cp,Na}$	N_{ba} (lb)	N_a (lb)
2.0	750.93	422.18	1.000	1.000	1.000	19348	34414

A_{Nc} (in^2)	A_{Nco} (in^2)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
612.00	324.00	1.000	1.000	1.000	1.000	13685	25849	0.70

ϕV_{cpg} (lb)
36189

11. Results

Interaction of Tensile and Shear Forces (Sec. 17.6.)

Tension	Factored Load, N_{ua} (lb)	Design Strength, øN_n (lb)	Ratio	Status
Steel	941	14528	0.06	Pass
Concrete breakout	**2027**	**11805**	**0.17**	**Pass (Governs)**
Adhesive	2027	16319	0.12	Pass

Shear	Factored Load, V_{ua} (lb)	Design Strength, øV_n (lb)	Ratio	Status
Steel	**2000**	**7556**	**0.26**	**Pass (Governs)**
|| Concrete breakout x-	8000	33834	0.24	Pass
Pryout	8000	36189	0.22	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. 17.6..2	0.00	0.26	26.5 %	1.0	Pass

SET-3G w/ 3/4"Ø F1554 Gr. 36 with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.

- Refer to manufacturer's product literature for hole cleaning and installation instructions.

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: 22046.0.ec6

DESCRIPTION: F2: Kegmen Monument

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	3.0	ksi
fy : Rebar Yield	=	60.0	ksi
Ec : Concrete Elastic Modulus	=	3,122.0	ksi
Concrete Density	=	145.0	pcf
φ Values Flexure	=	0.90	
Shear	=	0.750	

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	2.0	ksf
Soil Density	=	110.0	pcf
Increase Bearing By Footing Weight	=	No	
Soil Passive Resistance (for Sliding)	=	250.0	pcf
Soil/Concrete Friction Coeff.	=	0.30	

Increases based on footing Depth

Footing base depth below soil surface	=	ft
Allow press. increase per foot of depth	=	ksf
when footing base is below	=	ft

Increases based on footing plan dimension

Allowable pressure increase per foot of depth	=	ksf
when max. length or width is greater than	=	ft

Dimensions

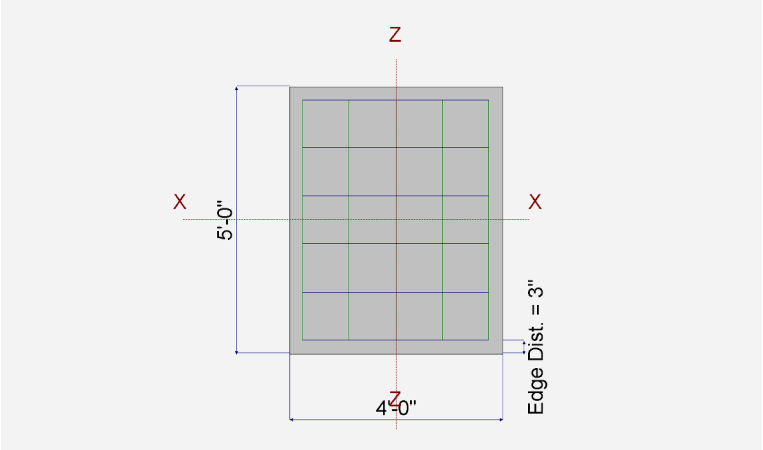
Width parallel to X-X Axis	=	4.0 ft
Length parallel to Z-Z Axis	=	5.0 ft
Footing Thickness	=	12.0 in

Pedestal dimensions...

px : parallel to X-X Axis	=	in
pz : parallel to Z-Z Axis	=	in
Height	=	in

Rebar Centerline to Edge of Concrete...

at Bottom of footing	=	3.0 in



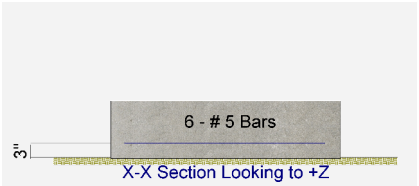
Reinforcing

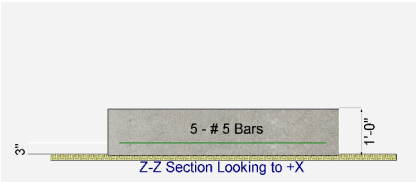
Bars parallel to X-X Axis			
Number of Bars	=		6
Reinforcing Bar Size	=	#	5
Bars parallel to Z-Z Axis			
Number of Bars	=		5
Reinforcing Bar Size	=	#	5

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation

	Bars along X-X Axis
# Bars required within zone	88.9 %
# Bars required on each side of zone	11.1 %





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	0.30							k
OB : Overburden	=								ksf
M-xx	=						2.80		k-ft
M-zz	=						0.930		k-ft
V-x	=						0.260		k
V-z	=						0.80		k

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: 22046.0.ec6

DESCRIPTION: F2: Kegmen Monument

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.1549	Soil Bearing	0.3097 ksf	2.0 ksf	+D+0.70E about X-X axis
PASS	1.905	Overturning - X-X	2.520 k-ft	4.80 k-ft	+0.60D+0.70E
PASS	4.610	Overturning - Z-Z	0.8330 k-ft	3.840 k-ft	+0.60D+0.70E
PASS	3.165	Sliding - X-X	0.1820 k	0.5760 k	+0.60D+0.70E
PASS	1.029	Sliding - Z-Z	0.560 k	0.5760 k	+0.60D+0.70E
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.01072	Z Flexure (+X)	0.1550 k-ft/ft	14.455 k-ft/ft	+1.20D+E
PASS	0.006364	Z Flexure (-X)	0.09199 k-ft/ft	14.455 k-ft/ft	+0.90D+E
PASS	0.03496	X Flexure (+Z)	0.5255 k-ft/ft	15.031 k-ft/ft	+0.90D+E
PASS	0.02603	X Flexure (-Z)	0.3913 k-ft/ft	15.031 k-ft/ft	+1.20D+E
PASS	0.01112	1-way Shear (+X)	0.9137 psi	82.158 psi	+1.20D+E
PASS	0.006719	1-way Shear (-X)	0.5521 psi	82.158 psi	+0.90D+E
PASS	0.03233	1-way Shear (+Z)	2.656 psi	82.158 psi	+0.90D+E
PASS	0.02430	1-way Shear (-Z)	1.996 psi	82.158 psi	+0.90D+E
PASS	0.007676	2-way Punching	1.261 psi	164.317 psi	+1.40D

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	2.0	n/a	0.0	0.160	0.160	n/a	n/a	0.080
X-X, +0.60D	2.0	n/a	0.0	0.0960	0.0960	n/a	n/a	0.048
X-X, +D+0.70E	2.0	n/a	9.450	0.01031	0.3097	n/a	n/a	0.155
X-X, +D+0.5250E	2.0	n/a	7.088	0.04773	0.2723	n/a	n/a	0.136
X-X, +0.60D+0.70E	2.0	n/a	15.750	0.0	0.2676	n/a	n/a	0.134
Z-Z, D Only	2.0	0.0	n/a	n/a	n/a	0.160	0.160	0.080
Z-Z, +0.60D	2.0	0.0	n/a	n/a	n/a	0.0960	0.0960	0.048
Z-Z, +D+0.70E	2.0	3.124	n/a	n/a	n/a	0.09815	0.2219	0.111
Z-Z, +D+0.5250E	2.0	2.343	n/a	n/a	n/a	0.1136	0.2064	0.103
Z-Z, +0.60D+0.70E	2.0	5.206	n/a	n/a	n/a	0.03415	0.1579	0.079

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
X-X, D Only	None	0.0 k-ft	Infinity	OK
X-X, +0.60D	None	0.0 k-ft	Infinity	OK
X-X, +D+0.70E	2.520 k-ft	8.0 k-ft	3.175	OK
X-X, +D+0.5250E	1.890 k-ft	8.0 k-ft	4.233	OK
X-X, +0.60D+0.70E	2.520 k-ft	4.80 k-ft	1.905	OK
Z-Z, D Only	None	0.0 k-ft	Infinity	OK
Z-Z, +0.60D	None	0.0 k-ft	Infinity	OK
Z-Z, +D+0.70E	0.8330 k-ft	6.40 k-ft	7.683	OK
Z-Z, +D+0.5250E	0.6248 k-ft	6.40 k-ft	10.244	OK
Z-Z, +0.60D+0.70E	0.8330 k-ft	3.840 k-ft	4.610	OK

All units k

Sliding Stability

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
X-X, D Only	0.0 k	0.960 k	No Sliding	OK
X-X, +0.60D	0.0 k	0.5760 k	No Sliding	OK
X-X, +D+0.70E	0.1820 k	0.960 k	5.275	OK
X-X, +D+0.5250E	0.1365 k	0.960 k	7.033	OK
X-X, +0.60D+0.70E	0.1820 k	0.5760 k	3.165	OK
Z-Z, D Only	0.0 k	0.960 k	No Sliding	OK
Z-Z, +0.60D	0.0 k	0.5760 k	No Sliding	OK
Z-Z, +D+0.70E	0.560 k	0.960 k	1.714	OK
Z-Z, +D+0.5250E	0.420 k	0.960 k	2.286	OK
Z-Z, +0.60D+0.70E	0.560 k	0.5760 k	1.029	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

Project File: 22046.0.ec6

DESCRIPTION: F2: Kegmen Monument

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	0.06563	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.40D	0.06563	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D	0.05625	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D	0.05625	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +0.90D	0.04219	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +0.90D	0.04219	-Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D+E	0.5086	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +1.20D+E	0.3913	-Z	Top	0.2592	AsMin	0.3875	15.031	OK
X-X, +0.90D+E	0.5255	+Z	Bottom	0.2592	AsMin	0.3875	15.031	OK
X-X, +0.90D+E	0.3745	-Z	Top	0.2592	AsMin	0.3875	15.031	OK
Z-Z, +1.40D	0.0420	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.40D	0.0420	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D	0.0360	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D	0.0360	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +0.90D	0.0270	-X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +0.90D	0.0270	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+E	0.08299	-X	Top	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +1.20D+E	0.1550	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +0.90D+E	0.09199	-X	Top	0.2592	AsMin	0.3720	14.455	OK
Z-Z, +0.90D+E	0.1460	+X	Bottom	0.2592	AsMin	0.3720	14.455	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	0.24 psi	0.24 psi	0.34 psi	0.34 psi	0.34 psi	82.16 psi	0.00	OK
+1.20D	0.21 psi	0.21 psi	0.29 psi	0.29 psi	0.29 psi	82.16 psi	0.00	OK
+0.90D	0.16 psi	0.16 psi	0.22 psi	0.22 psi	0.22 psi	82.16 psi	0.00	OK
+1.20D+E	0.50 psi	0.91 psi	1.98 psi	2.58 psi	2.58 psi	82.16 psi	0.03	OK
+0.90D+E	0.55 psi	0.86 psi	2.00 psi	2.66 psi	2.66 psi	82.16 psi	0.03	OK

Two-Way "Punching" Shear

All units k

Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	1.26 psi	164.32 psi	0.007676	OK
+1.20D	1.08 psi	164.32 psi	0.006579	OK
+0.90D	0.81 psi	164.32 psi	0.004935	OK
+1.20D+E	1.09 psi	164.32 psi	0.00661	OK
+0.90D+E	0.84 psi	164.32 psi	0.005123	OK